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Lecture 07

Introduction to Bridge Engineering

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CE 416: Reinforced Concrete Design - II



Lecture Contents

- General
- Design of Bridges
- Design of Simply Supported RC Slab Bridges
- Example 7.1
- Famous Bridges in the World
- References

Learning Outcomes

□ At the end of this lecture, students will be able to;

- > **Differentiate** among different type of bridges.
- Learn AASHTO design loads for bridge design
- > Analyze & Design Simply Supported RC Slab Bridges



□ Introduction

 Bridge is a structure having an opening not less than 6000 mm that forms part of a highway or over, or under which the highway passes.







□ Introduction

- Structures having span less than 6000mm are generally called culverts.
- Culvert is a tunnel structure constructed under roadways or railways to provide cross drainage.









Introduction

- Bridge is the key element in a Transportation System
 - It controls the capacity of the system.
 - It is the highest cost per mile of the system.
 - If the bridge fails, the system fails.





Components of Bridge

- A bridge consists of super structure and sub structure.
 - 1. Super structure

Structural parts of the bridge which provides the horizontal span

e.g., girder and deck.

2. Sub structure

Structural parts of the bridge which supports the horizontal span.

e.g., pier cap, pier pile cap etc.

• These components are shown in figure on the next slide.



Components of Bridge





Components of Bridge





Classification of Bridges

- Bridges can be classified according to:
 - Materials (concrete, steel or wood etc),
 - Usage (pedestrian, highway, or railroad),
 - Span (short, medium, or long),
 - Structural form (slabs, girder, truss, arch, suspension, or cable-stayed).





- It is, however, suitable to classify bridges according to the location of the main structural elements relative to the surface on which the user travels
 - 1. Main structure **Below** the deck line,
 - 2. Main structure Above the deck line, or
 - 3. Main structure Coinciding with the deck line.





- 1. Main structure Below the deck line
 - Arched, Truss-Arched Bridges, Masonry Arch, Concrete Arch, the Steel-Truss Arch, and the Steel Deck Truss.



Masonry Arch



Masonry Arch



Steel Deck Truss Bridge





1. Main structure Below the deck line



Concrete Arch



Steel Truss Arch





- 2. Main structure Above the deck line
 - Suspension, Cable Stayed, and Through-Truss bridges are included in this category



Suspension: Deck is supported by two main cables through secondary cables (hangers).





2. Main structure Above the deck line



Cable-stayed: Deck is supported by tower directly through cables.





2. Main structure Above the deck line



Through Bridges



□ Classification of Bridges

- 3. Main structure Coinciding with the deck line
 - Girder bridges of all types are included in this category.
 Examples are:
 - Slab (solid and voided),
 - T-beam and I-beam,
 - Wide-flange beam,
 - Concrete box girder,
 - Steel box,
 - Steel plate girder.



Concrete Box Girder Bridge





3. Main structure Coinciding with the deck line



Box Girder Bridge

Girder Bridge Under Construction





3. Main structure Coinciding with the deck line





- Loads to be considered in bridge design can be divided into two broad categories:
 - 1. Permanent loads
 - 2. Transient loads.



- 1. Permanent Loads
 - Self weight of girders and deck, wearing surface, curbs and parapets and railings, utilities and luminaries and pressures from earth retainment.
 - Two important dead loads are:
 - i. DC: Dead load of structural components and nonstructural attachments.
 - ii. DW: Dead load of wearing surface.



- 1. Permanent Loads
 - Material Properties for Pavement
 - Unit weight of Bitumen, $\gamma_B = 140 lb/cft$
 - Unit weight of Concrete, $\gamma_c = 150 lb/cft$
 - Load factors for Pavement Dead Loads
 - The maximum load factor for DC = 1.25
 - The maximum load factor for DW = 1.5



- 2. Transient Loads
 - Gravity (Live) loads due to vehicular, railway and pedestrian traffic.
 - The automobile is one of the most common vehicular live load on most bridges; it is the truck that causes the critical load effects.
 - Lateral loads due to water, wind, earthquake and ship collisions etc.



- 2. Transient Loads
 - Following effects caused by Live load are also very important and must be considered in the design of a bridge.
 - Impact (dynamic effects),
 - Braking forces,
 - Centrifugal forces (if present) and
 - The effects of other trucks simultaneously present.

Vehicular Design Loads

- The vehicle combination as described in AASHTO (1994) LRFD Bridge specifications are designated as HL-93 for Highway Loading accepted in 1993.
- The AASHTO design loads model consists of three distinctly different loads
 - 1. Design Truck,
 - 2. Design Tandem,
 - 3. Design Lane.





Vehicular Design Loads

• Design Truck







- Vehicular Design Loads
 - ✤ Design Tandem





Vehicular Design Loads

- ✤ Design Lane load
 - The AASHTO design lane loading is like a caravan of trucks.
 - It is 0.064 k/ft² and is assumed to occupy a region of 10 ft.
 - It is applied as 0.064 k/ft² (64 lb/ft²) of pressure to a width of 10 ft over the entire length of bridge for FEM.





Vehicular Design Loads

- Summary
 - In summary three design loads should be considered;

(i) Design truck (ii) Design tandem and (iii) Design lane

- These loads are superimposed by two ways to yield the live load effects, which are combined with the other load effects.
- The two ways of superposition are:
 - Truck + lane
 - Tandem + lane



Load Modifier

- A factor accounting for ductility, redundancy and the operational importance of the bridge.
- It is taken as 1.05 for simply supported bridges and is applied on already factored values of bending moments.

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Design of Simply Supported RC Slab Bridges

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Simply Supported RC Slab Bridge

- This type of bridge consist of only a slab (without any other supporting member such as girders).
- A slab bridge is widely used when the bridge crosses a minor road or small river.



Design Lane

- Slab bridges can be analyzed as 3D, 2D and 1D models.
- If it is to be design as ID model, then it is called Line Analysis.
- Line Analysis is the simplest of all methods, which has been described in subsequent slides.

Design Lane

- Steps in Line Analysis
 - The bridge deck is divided into various strips having width "E" called design lanes
 - These design lanes are then transformed to line elements (1D model) for line analysis.
 - 3. The moment are calculated from line analysis.
 - 4. These moments (M) are then divided by the design lane width (E) to get moment per foot (M/E) for the slab.

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Analysis and Design of Simply Supported RC Slab Bridges







Design Lane

- Calculation of Design Lane Width
 - Design lane widths can be calculated as;

 $E = Smaller of E_s and E_m$

Where;

 $E_s = lane width for Single lane loaded$

 $E_m = lane width for multi lane loaded$

• The procedure of determing E_s and E_m is described in next slides
Design Lane

- Calculation of Design Lane Width
 - For single lane loaded

 $E_s = 10 + 5\sqrt{L_1 W_1'}$ (in) – (i)

Where;

- L_1 = Modified span length = Minimum of span length "S" and 60 ft
- W_1' = Modified edge to edge width

= Minimum of (Overall width of bridge, W_1) or 30 ft





Design Lane

- Calculation of Design Lane Width
 - For multi lane loaded

$$E_m = 84 + 1.44\sqrt{L_1 W_m} \le \frac{W_m}{N_L}$$
 (in) ----- (ii)

Where;

 $N_L = No$

- L_1 = Same as single lane loaded case = Minimum of (S) and 60 ft
- W_m = Minimum of (overall width of bridge, W₁) or 60 ft

of design lanes = INT(W/12)

$$W_1$$



Thickness of Bridge Slab

Minimum thickness of bridge slab is given by;

 $h_{min} = \frac{1.2(S+10)}{30}$ (ft)

where;

S = span of bridge (ft)





Flexural Analysis

LL+IM

Maximum factored bending moment per unit feet is given by; \mathbf{O}

$$M_u = 1.05 (1.25M_{DC} + 1.5M_{DW} + 1.75M_{LL+IM})$$

Where;

$$M_{DC} = W_{DC}S^{2}/8 \quad (ft-kip/ft) \quad (W_{DC} = h\gamma_{concrete})$$

$$M_{DW} = W_{DW}S^{2}/8 \quad (ft-kip/ft) \quad (W_{DW} = h\gamma_{wearing surface})$$

$$M_{LL+IM} = 1.33(M_{Tandem} \text{ OR } M_{Truck}) + M_{lane} \quad (ft-kip)$$

$$Live Load$$

' ''lane

Convert M_{LL+IM} to ft-kip/ft, Divide M_{LL+IM} by "E", design lane width.

Flexural Analysis

- Slab Moments due to Live Loads
 - a) Moment due to HL-93 Truck load, M_{truck}
 - Max. moment due to truck load can be obtained by placing the middle axle at mid span of the bridge and rear axle load at a distance of 14 ft from the middle axle load.



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Analysis and Design of Simply Supported RC Slab Bridges

Flexural Analysis

- Slab Moments due to Live Loads
 - b) Moment due to Tandem load, M_{tandem}
 - Maximum moment due to tandem load can be obtained by placing the two loads at 2 ft from the mid span.
 - Solving the given system gives





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Analysis and Design of Simply Supported RC Slab Bridges

Flexural Analysis

- Slab Moments due to Live Loads
 - b) Moment due to Design lane load, M_{Lane}
 - The maximum bending moment due to design lane load is given by





Determination of Reinforcement

1. Bottom longitudinal reinforcement

$$a = d - \sqrt{d^2 - \frac{2.614M_u}{f_c'b_w}} \quad and \quad A_s = \frac{M_u}{\varphi f_y \left(d - \frac{a}{2}\right)}$$

2. Bottom Transverse reinforcement (A5.14.4.1)

$$A_{tr} = Smaller \ of \left(\frac{A_s}{\sqrt{S}}, \frac{A_s}{2}\right)$$
 but not less than A_{S+T}

Where;

 $A_{S+T} = 0.0018A_g$

- Determination of Reinforcement
 - 3. Top longitudinal reinforcement

 $A_{S+T} = 0.0018A_g$

 $S_{max} = lesser \ of \ 5h \ or \ 18''$

4. Top Transverse reinforcement (distribution reinforcement) $A_{S+T} = 0.0018A_g$

 $S_{max} = lesser of 5h or 18''$

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Analysis and Design of Simply Supported RC Slab Bridges

Reinforcement Detailing





Problem Statement

- A simply supported RC slab bridge shown in Figure on the next slide, with span length of 35' center to center of bearings. The Roadway width is 44 ft curb to curb. Allow for a future wearing surface of 3" thick bituminous overlay. The material strengths are $f_c' = 4$ ksi and $f_y = 60$ ksi
 - a) Analyze the RC slab using HL-93 live load model for live loads.
 - **b) Design** the bridge slab using #8 bar only.

Problem Statement



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Solution

- Given Data
 - Span, S = 35'
 - W = 44'
 - $W_1 = 46.5'$

Wearing surface, $h_{WS} = 3$ "

- $f_c' = 4ksi$ and $f_y = 60ksi$.
- Required Data
 - *a) Analyze* the RC slab using HL-93 live load model for live loads.
 - **b) Design** the bridge slab using #8 bar only.





Solution

Step 1: Selection of sizes

Minimum thickness of bridge slab is given by formula

$$h_{min} = \frac{1.2(10+S)}{30}$$

Substituting value of S , gives

$$h_{min} = \frac{1.2(10+35)}{30} = 1.8' \text{ or } 21.6'$$

Take h = 22"



Solution

Step 2: Calculation of loads

Self weight of slab and wearing surface is calculated as follows;

$$W_{DC} = \gamma_c h = 0.150 \times \left(\frac{22}{12}\right) = 0.275 ksf$$

And

$$W_{DW} = \gamma_W h = 0.140 \times \left(\frac{3}{12}\right) = 0.035 ksf$$



Solution

- Step 3: Analysis
 - b) Dead load Moments

$$M_{DC} = \frac{W_{DC}S^2}{8} = \frac{0.275 \times 35^2}{8} = 42kip.ft$$

And

$$M_{DW} = \frac{W_{DC}S^2}{8} = \frac{0.035 \times 35^2}{8} = 5.33 kip.ft$$



Solution

Step 3: Analysis

b) Live load Moments (HL-93 Live load model)

I. Truck Moment

 $A_y = 72 - \frac{36S - 336}{S} = 72 - \frac{36(35) - 336}{35} = 45.6 kip$

And

$$M_{truck} = A_y \left(\frac{S - 28}{2} + 14\right) - 448 = 45.6 \left(\frac{35 - 28}{2} + 14\right) - 448$$

$$M_{truck} = 350 kip.ft$$



Solution

Step 3: Analysis

b) Live load Moments (HL-93 Live load model)

II. Tandem moment

Moment due to tandem load is given by

 $M_{tandem} = 12S - 48$

By putting value of S, we get

 $M_{tandem} = 12(35) - 48$

 $M_{tandem} = 372 kip.ft$



Solution

Step 3: Analysis

b) Live load Moments (HL-93 Live load model)

III. Lane load moment

Moment due to Lane load is given by

$$M_{Lane} = \frac{0.64S^2}{8} = \frac{0.64(35)^2}{8}$$

$$M_{Lane} = \frac{0.64S^2}{8} = 98kip.ft$$



□ Solution

Step 3: Analysis

b) Live load Moments (HL-93 Live load model)

Now,

 $M_{LL+IM} = 1.33[larger of M_{tandem} or M_{truck}] + M_{Lane}$

Since $M_{tandem} > M_{truck}$, therefore we will use M_{tandem}

 $M_{LL+IM} = 1.33[372] + 98$

 $M_{LL+IM} = 592.76 kip.ft$

To convert M_{LL+IM} to moment/ft, Divide MLL+IM by "E" design lane width.



Solution

Step 3: Analysis

b) Live load Moments (HL-93 Live load model)

Calculation of Design Lane width "E"

$$\begin{split} E_{s} &= 10 + 5\sqrt{L_{1}W_{s}} \ (in) \\ E_{s} &= 10 + 5\sqrt{35 \times 30} = 172'' \\ \text{And} \\ E_{m} &= 84 + 1.44\sqrt{L_{1}W_{m}} \ \leq \frac{W_{m}}{N_{L}} \ (in) \\ E_{m} &= 142'' \ \leq 186'' \\ \text{Now,} \end{split}$$
 $L_{1} &= \text{Min. of } S = 35' \text{ and } 60' = 35' \\ W_{s} &= \text{Min. of } W_{1} = 46.5' \text{ or } 30' = 30' \\ W_{m} &= \text{Min. of } W_{1} = 46.5' \text{ or } 60' = 46.5' \\ \text{and} \\ N_{L} &= \text{INT} (W/12) = \text{INT} (44/12) = 3 \end{split}$

$$E = min. of E_s and E_m = 142'' or 11.84'$$



Solution

- Step 3: Analysis
 - b) Live load Moments (HL-93 Live load model)

Hence the M_{LL+IM} per foot is determined as;

$$M_{LL+IM}(per\ foot) = \frac{M_{LL+}}{E} = \frac{592.76}{11.84} = 50 kip.ft$$

Total Factored Moment

 $M_u = 1.05 (1.25M_{DC} + 1.5M_{DW} + 1.75M_{LL+I})$ = 1.05 (1.25 × 42 + 1.5 × 5.33 + 1.75 × 50) $M_u = 155.39kip.ft \text{ or } 1864.8in.kip$



□ Solution

- Step 4: Determination of reinforcement
- 1. Bottom Longitudinal Reinforcement

$$a = d - \sqrt{d^2 - \frac{2.614M_u}{f_c'b_w}} = 20.5 - \sqrt{20.5^2 - \frac{2.614 \times 1864.8}{4 \times 12}} = 2.64''$$

Now,

$$A_{s} = \frac{M_{u}}{\emptyset f_{y} \left(d - \frac{a}{2} \right)} = \frac{1864.8}{0.9 \times 60 \left(20.5 - \frac{2.64}{2} \right)} = 1.8in^{2}/ft$$

Provide #8@4" c/c

$$S_{max} = 3h \text{ or } 18'' = 18'' > provided spacing \rightarrow OK!$$



Solution

- Step 4: Determination of reinforcement
- 2. Bottom Transverse Reinforcement

 $A_{tr} = Smaller \ of \left(\frac{A_s}{\sqrt{S}}, \frac{A_s}{2}\right)$ but not less than $A_{s,min}$

By putting values, we get

$$A_{tr} = Smaller \ of \left(\frac{1.8}{\sqrt{35}}, \frac{1.8}{2}\right) = 0.30 in^2/ft$$

 $A_{s,min} = 0.0018A_g = 0.0018 \times 12 \times 22 = 0.476in^2/ft > A_{tr}$

Take $A_{tr} = 0.476 i n^2 / ft$ (Provide #5 @ 7.5" c/c)



Solution

- Step 4: Determination of reinforcement
- 3. Top longitudinal reinforcement

 $A_{S+T} = 0.0018A_g = 0.476in^2/ft$

Provide #5 @ 7.5" c/c

 $S_{max} = 5h \text{ or } 18'' = 18'' > provided spacing \rightarrow OK!$

4. Top Transverse reinforcement (distribution reinforcement) $A_{S+T} = 0.0018A_g = 0.476in^2/ft$

Provide #5 @ 7.5" c/c

 $S_{max} = 5h \text{ or } 18'' = 18'' > provided spacing \rightarrow OK!$



Solution

Step 5: Drafting



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Some Famous Bridges in The World

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Longest Bridge



Danyang-Kunshan Grand Bridge in China (164800 m)



Longest Span



Akashi Kaikyō Bridge, Japan (1991 m)





Highest Bridge



Si Du River Bridge (472 m high)





Bridges in Pakistan



Jamshoro Bridge, Jamshoro



Bridges in Pakistan



Attock Bridge, Attock





Bridges in Pakistan



Malir Bridge, Karachi Longest Bridge in Pakistan (5000 m)





Bridges in Pakistan



Chiniot Railway Bridge (1877)





Bridges in Pakistan



Bridge at Kallar Kahar, M2 Motorway

References



- Design of Highway Bridges by Richard M. Barker.
- Building Code Requirements for Structural Concrete (ACI 318-19)

