



## **Lecture 05**

# **Introduction to Earthquake Resistant Design of RC Structures (Part – I)**

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# Lecture Contents

- Introduction to Earthquake and Its Effects on Buildings
- Earthquake Design Philosophy
- Seismic Loading Criteria
- Static Lateral Force Procedure
- Example 5.1
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- Appendix



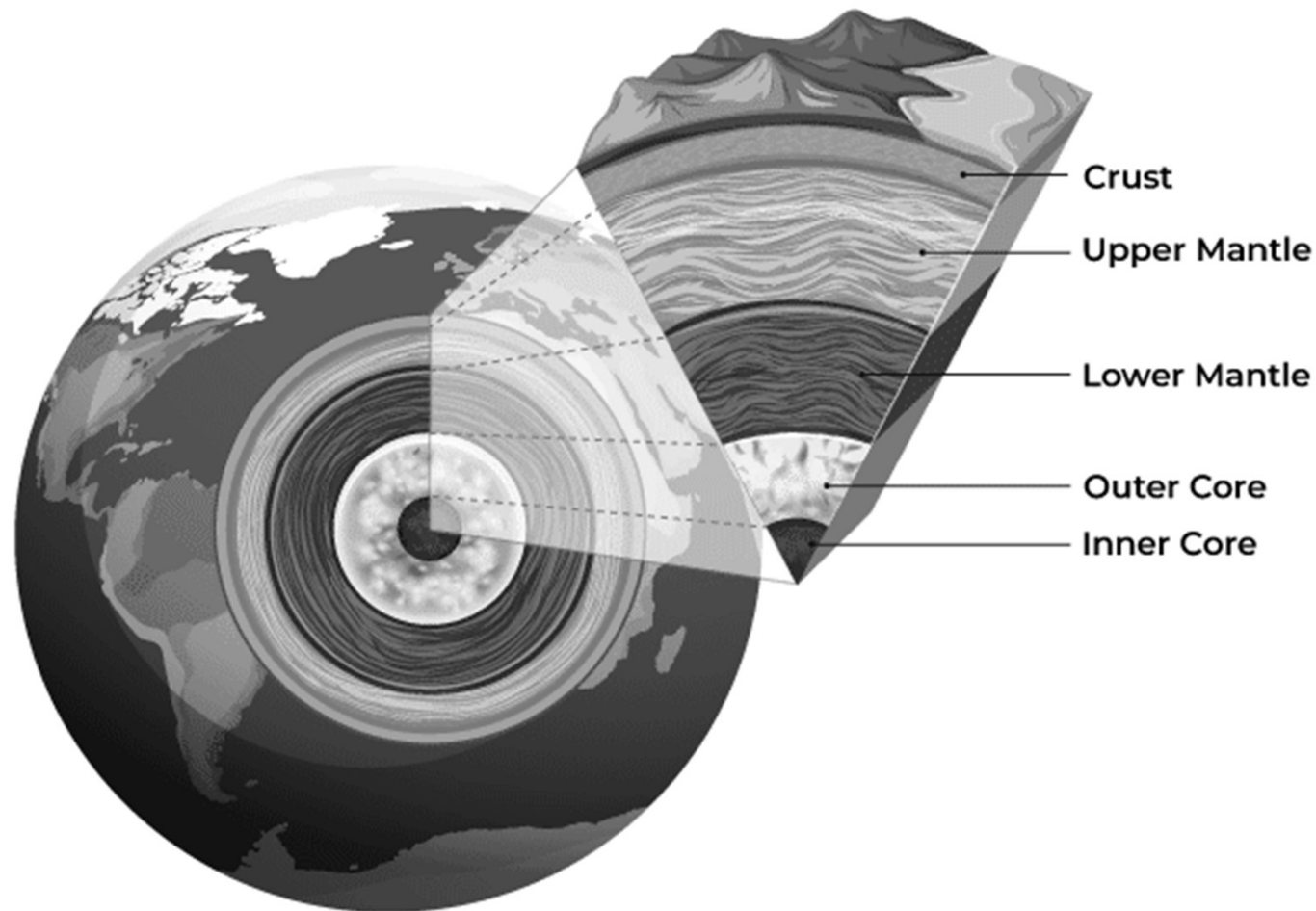
# Learning Outcomes

- ❑ **At the end of this lecture, students will be able to;**
  - ***Describe*** effects of earthquake loading on buildings
  - ***Explain*** earthquake design philosophy and seismic loading criteria
  - ***Calculate*** base shear using static lateral force procedures



# Introduction to Earthquake and Its Effects on Buildings

## □ The Earth's Interior

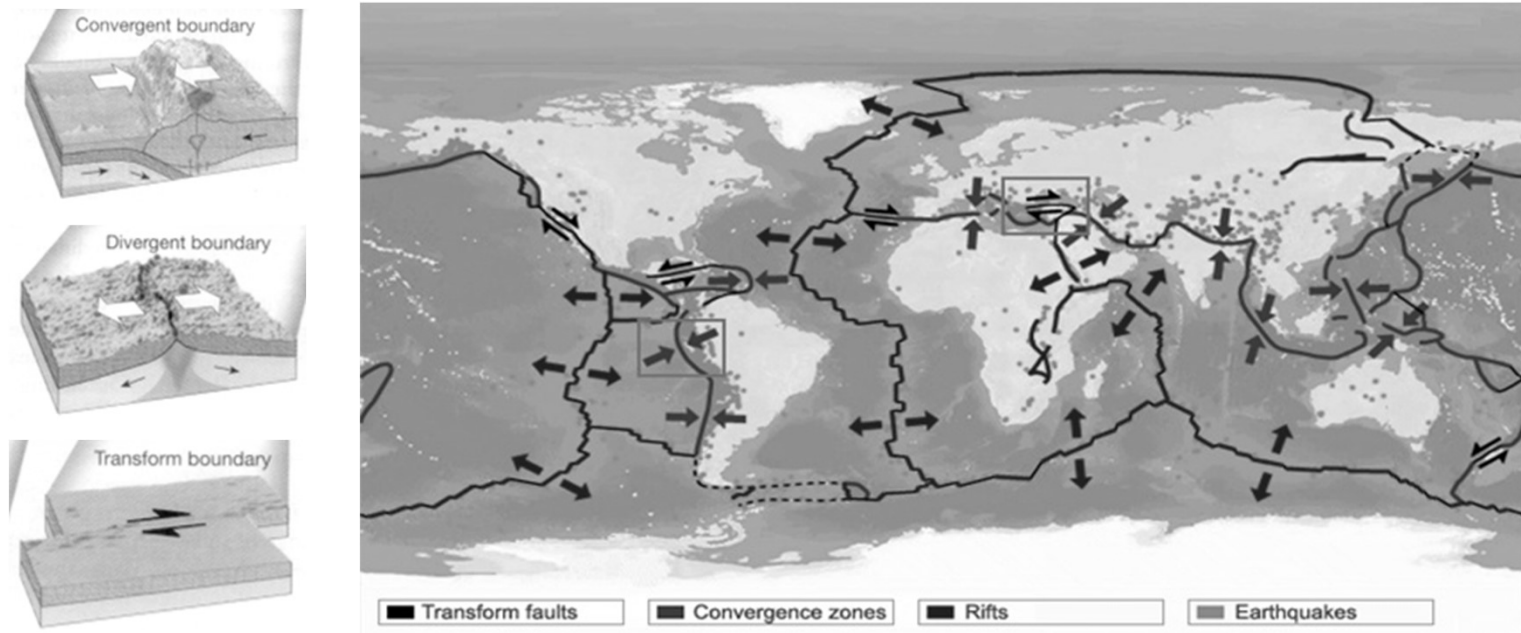




# Introduction to Earthquake and Its Effects on Buildings

## □ The Earth's Interior

- Earthquake results from the sudden movement of the tectonic plates in the earth's crust.

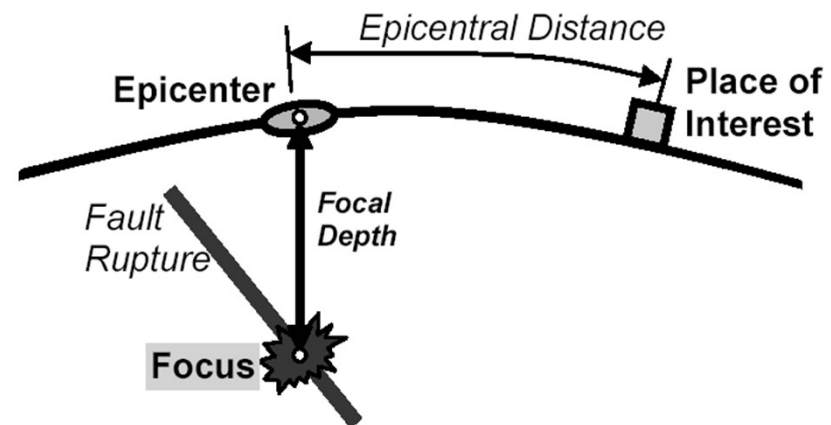
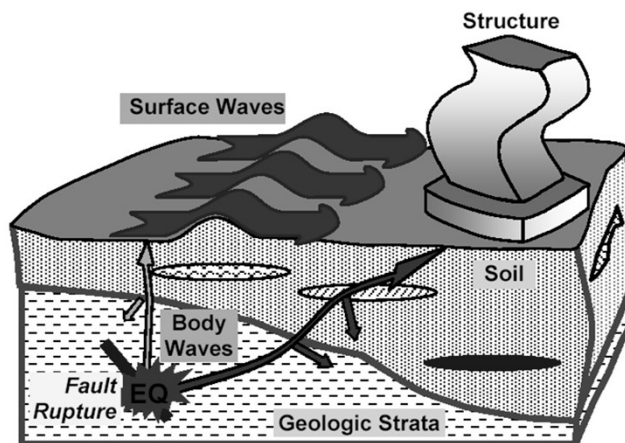




# Introduction to Earthquake and Its Effects on Buildings

## □ Effect of Earthquake

- The movement, taking place at the fault lines, causes energy release which is transmitted through the earth in the form of waves.
- These waves reach the structure causing shaking.

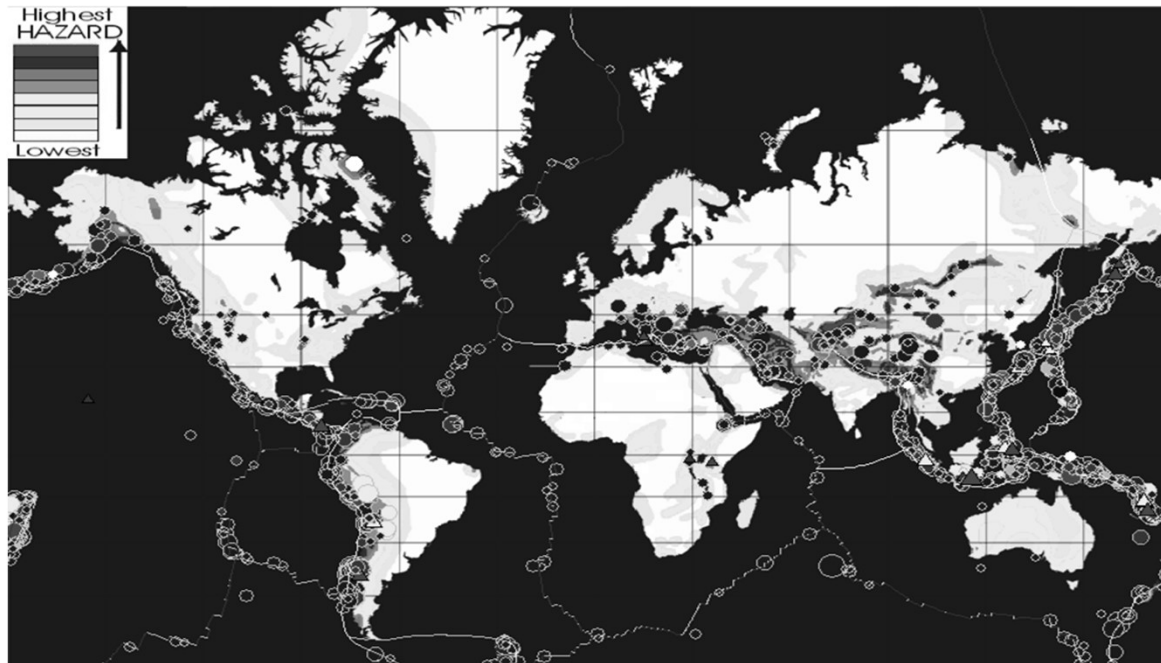




# Introduction to Earthquake and Its Effects on Buildings

## □ Seismic Events

- Seismic events around the globe are shown below
- These events mostly take place at boundaries of Tectonic plates

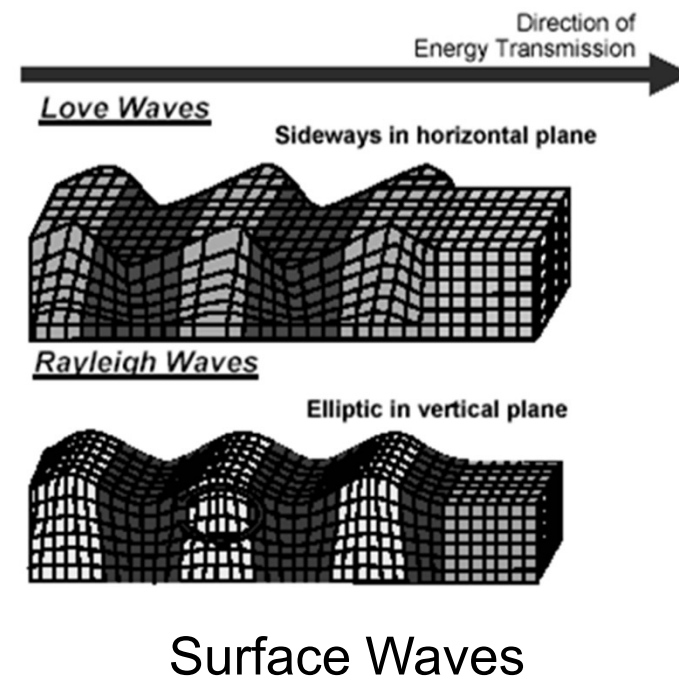
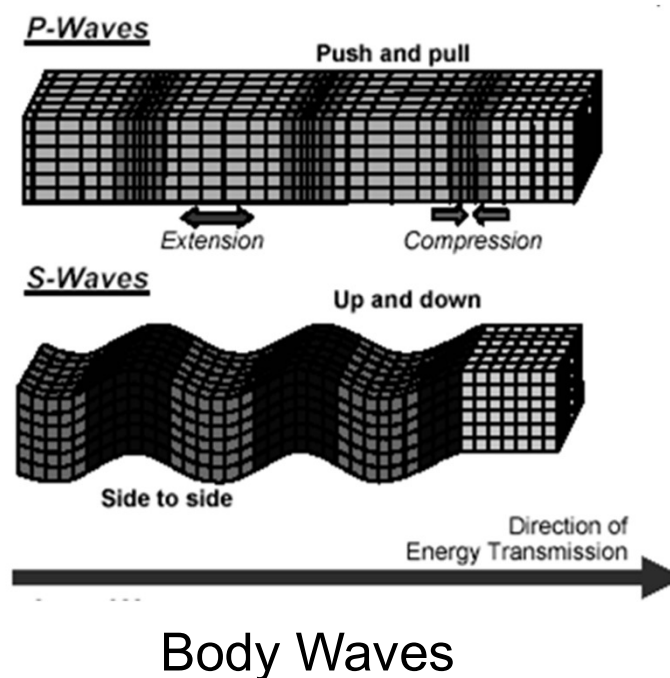


Dots represent an earthquake



# Introduction to Earthquake and Its Effects on Buildings

## □ Types of Waves Generated Due to Earthquake







# Introduction to Earthquake and Its Effects on Buildings

## ❑ Displacement due to Earthquake



Figure 1: Effect of Inertia in a building when shaken at its base

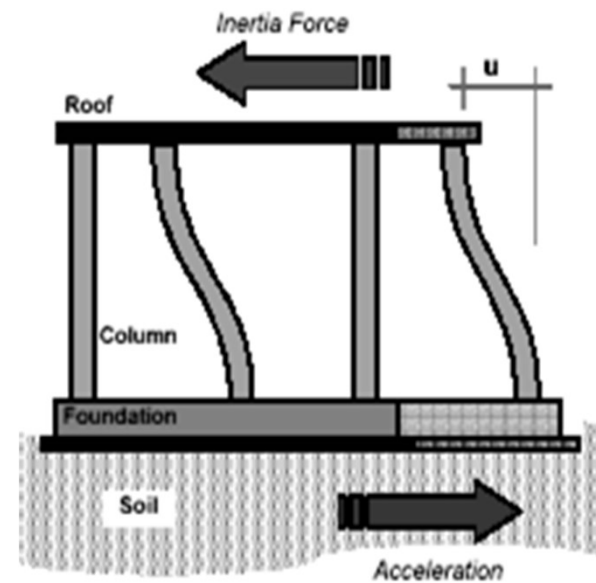


Figure 2: Inertia force and relative motion within a building



# Introduction to Earthquake and Its Effects on Buildings

## □ Horizontal and Vertical Shaking

- Earthquake causes shaking of the ground in all three directions.
- The structures designed for gravity loading (DL+LL) will be normally safe against vertical component of ground shaking.
- The vertical acceleration during ground shaking either adds to or subtracts from the acceleration due to gravity.
- The structures are normally designed for horizontal shaking to minimize the effect of damages due to earthquakes.

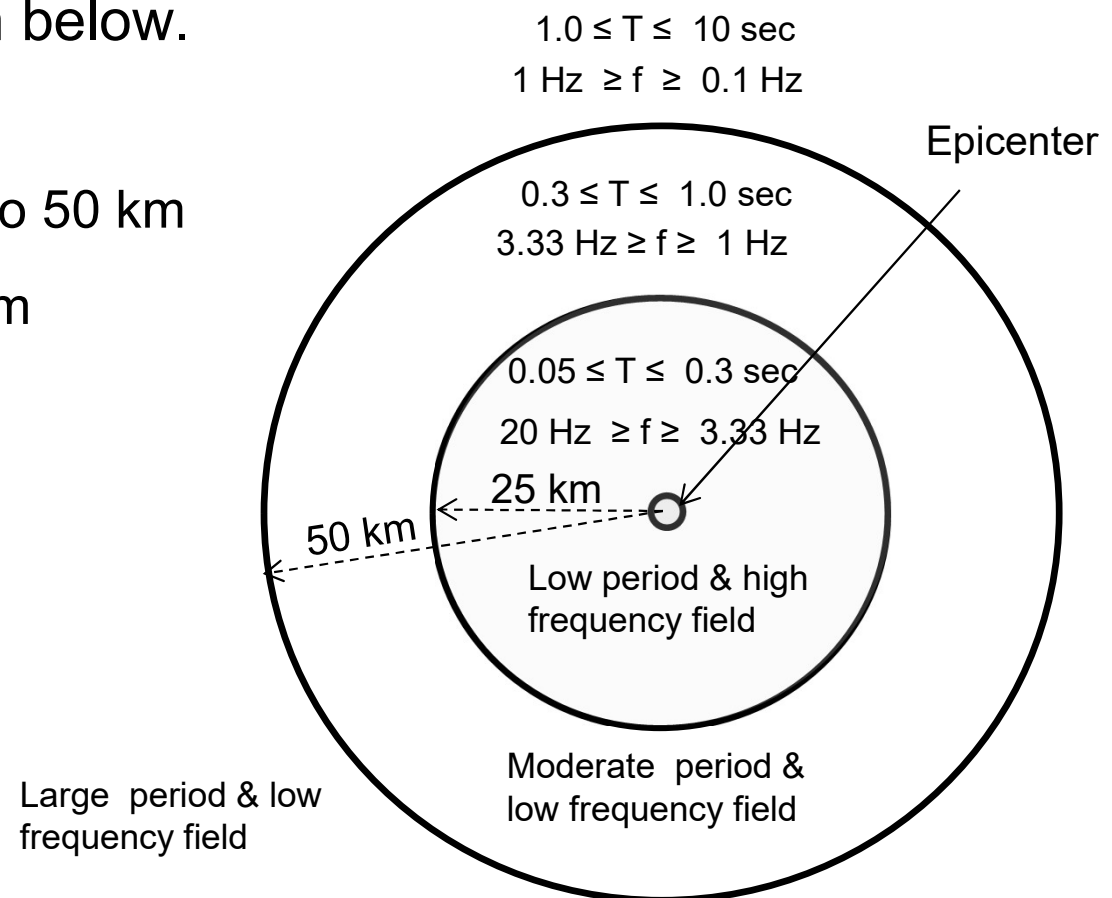


# Introduction to Earthquake and Its Effects on Buildings

## □ Earthquake characteristics

- The characteristics of earthquake with respect to distance from the epicenter are shown below.

1. Near Field: 0 to 25 km
2. Intermediate Field: 25 to 50 km
3. Far Field: Beyond 50 km

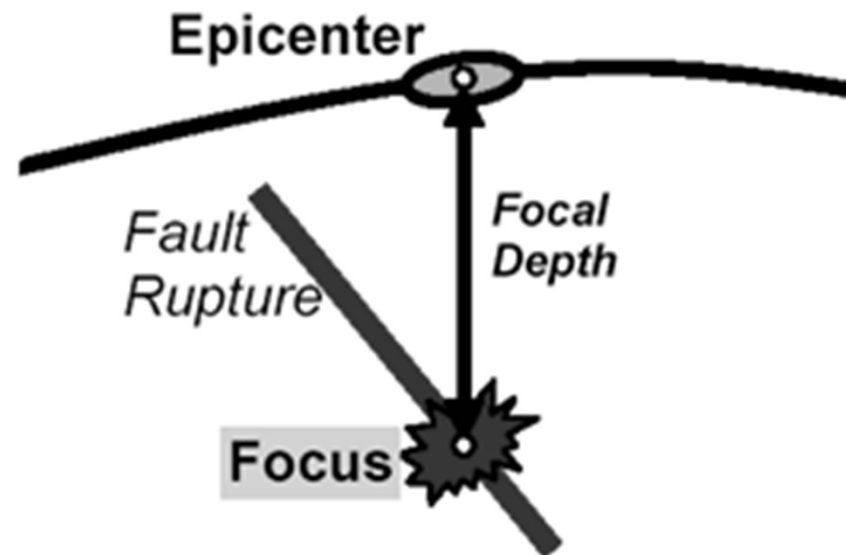




# Introduction to Earthquake and Its Effects on Buildings

## □ Types of earthquake based on focal depth

1. **Shallow earthquake:** Depth of focus varies between 0 and 70 km.
2. **Deep earthquake:** Depth of focus varies between 70 and 700 km.





# Introduction to Earthquake and Its Effects on Buildings

## □ Resonance risk for structures

- The natural time period of a structure is its important characteristic to predict behavior during an earthquake of certain time period (Resonance phenomenon).
- For a particular structure, the natural time period is a function of mass and stiffness.

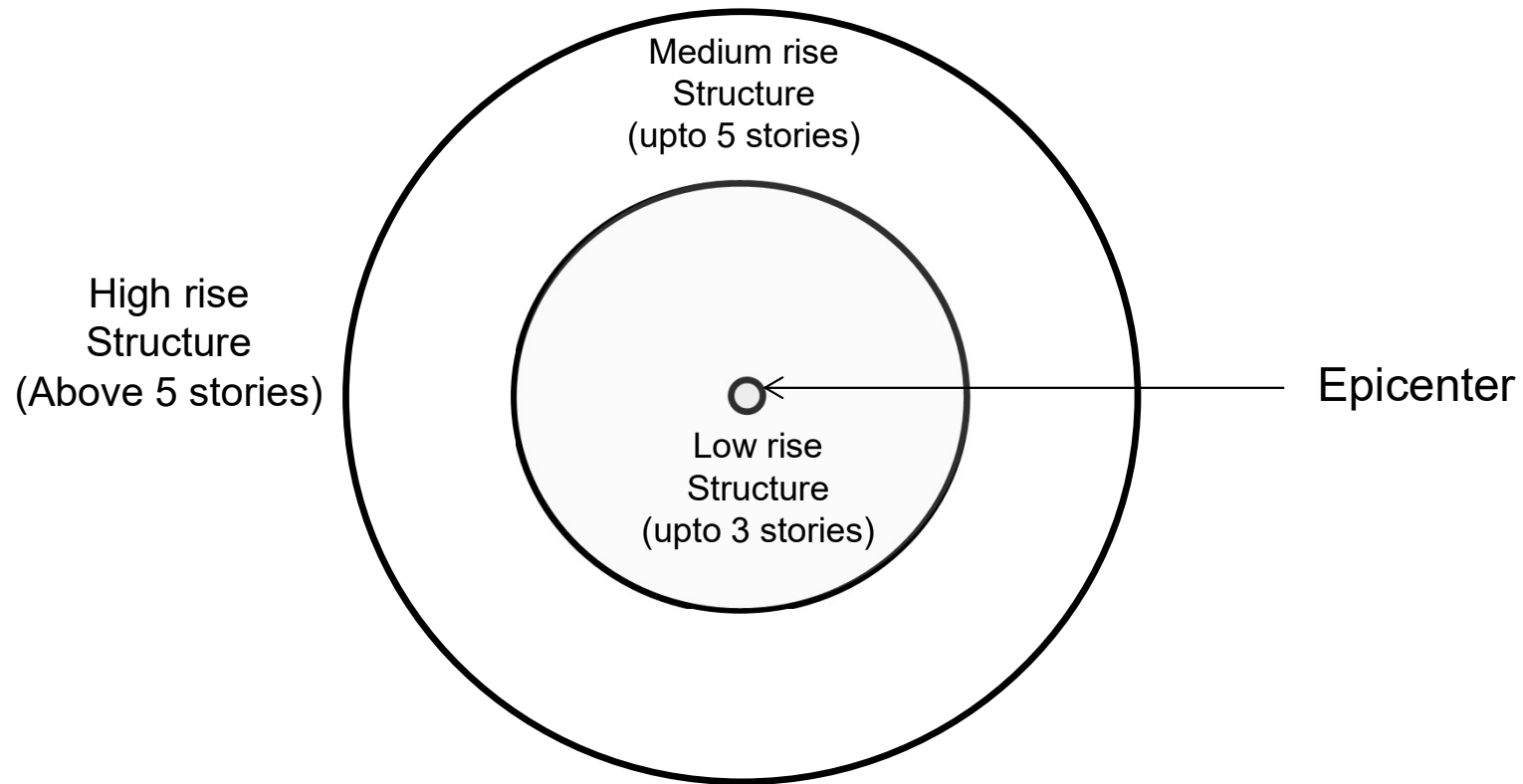
$$T = 2\pi\sqrt{\frac{m}{k}}$$

- “T” can be roughly estimated from:  $T = 0.1 \times \text{number of stories}$



# Introduction to Earthquake and Its Effects on Buildings

- ❑ Resonance risk for structures near, intermediate and far field earthquakes



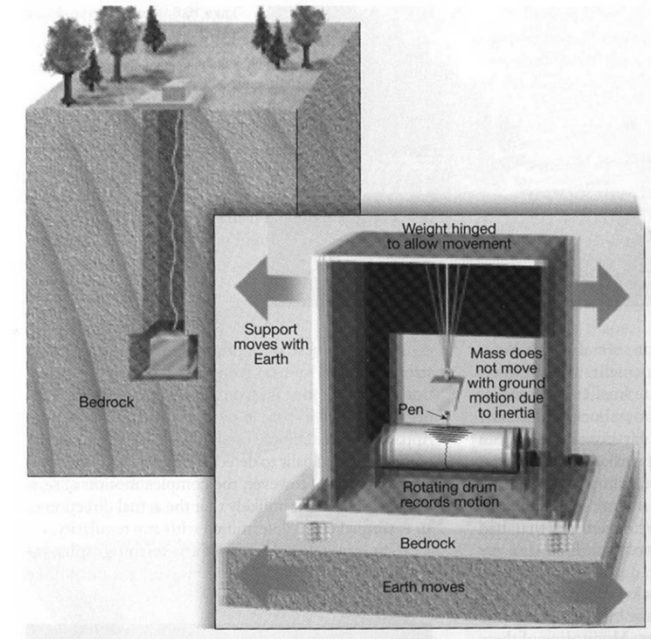


# Introduction to Earthquake and Its Effects on Buildings

## □ Earthquake Recording

### ❖ Seismograph

- Using multiple seismographs around the world, accurate location of the epicenter of the earthquake, as well as its magnitude or size can be determined.
- Working of seismograph shown in figure.



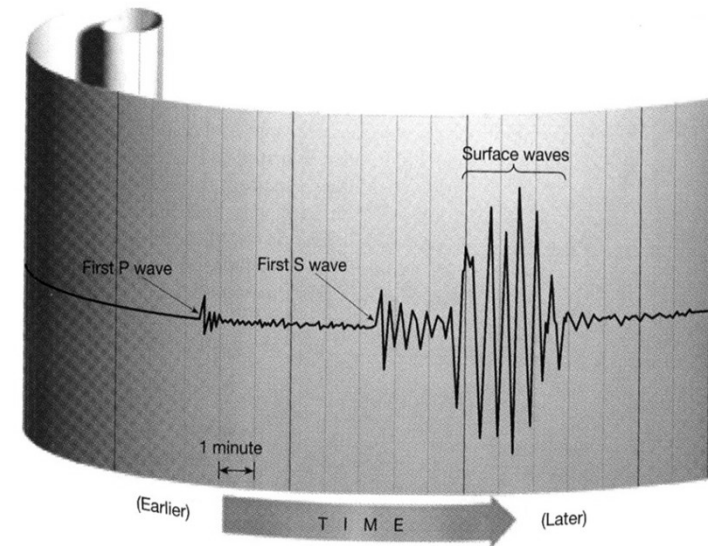


# Introduction to Earthquake and Its Effects on Buildings

## □ Earthquake Recording

### ❖ Richter Scale

- In 1935, Charles Richter (US) developed this scale.
- The Richter scale is logarithmic, So, a magnitude 5 Richter measurement is ten times greater than a magnitude 4; while it is  $10 \times 10$ , or 100 times greater than a magnitude 3 measurement.



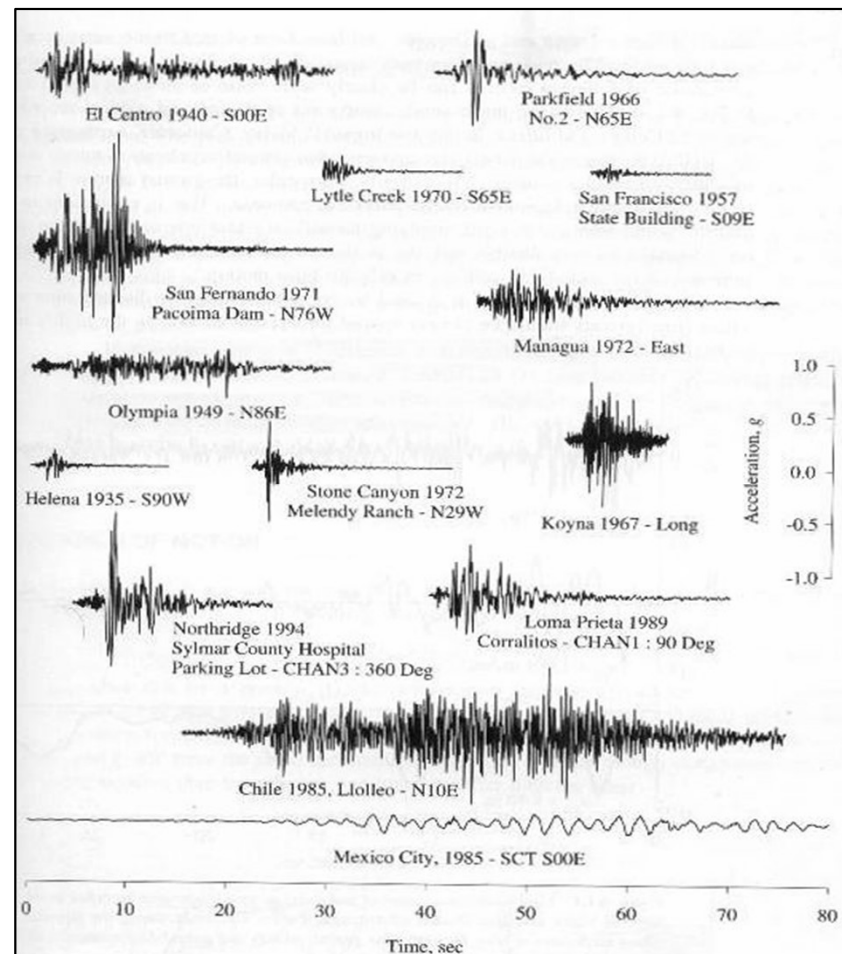




# Introduction to Earthquake and Its Effects on Buildings

## □ Earthquake Recording

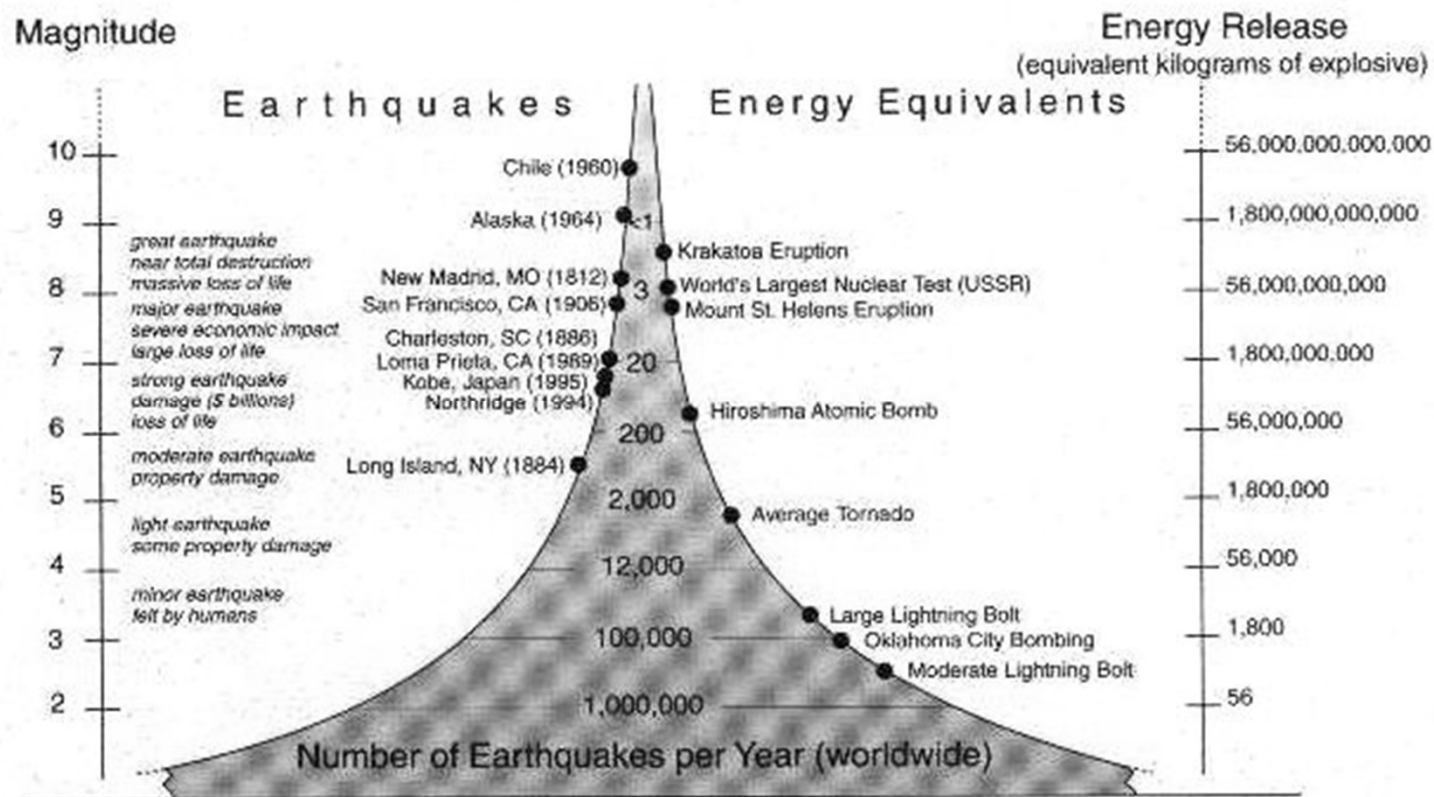
### ❖ Some Famous Earthquake Records





# Introduction to Earthquake and Its Effects on Buildings

## □ Earthquake Occurrence





# Introduction to Earthquake and Its Effects on Buildings

## □ Importance of Architectural Features

- The behavior of a building during earthquakes depend critically on its overall shape, size and geometry, in addition to how the earthquake forces are carried to the ground.
- At the planning stage, architects and structural engineers must work together to ensure that the unfavorable features are avoided, and a good building configuration is chosen

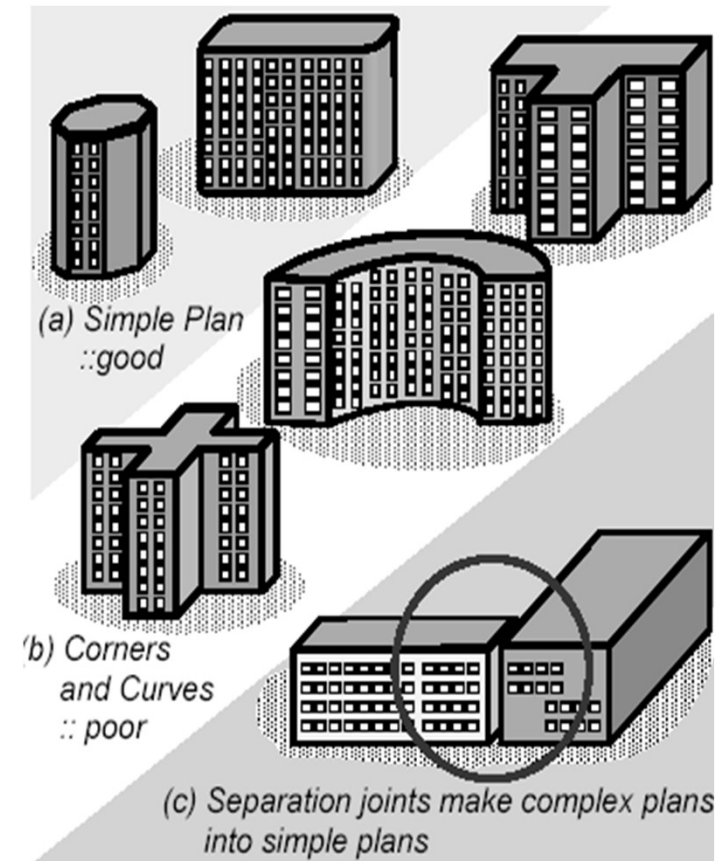
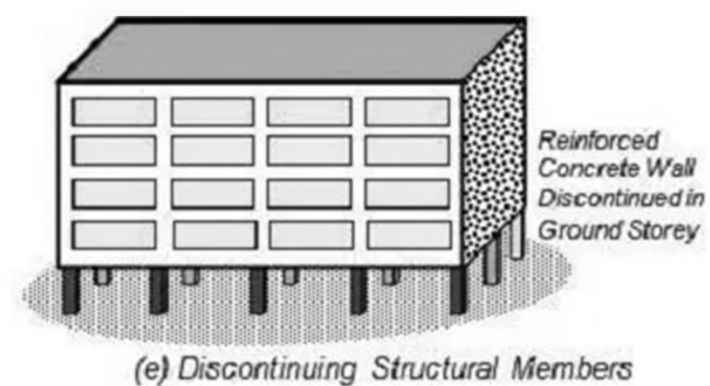
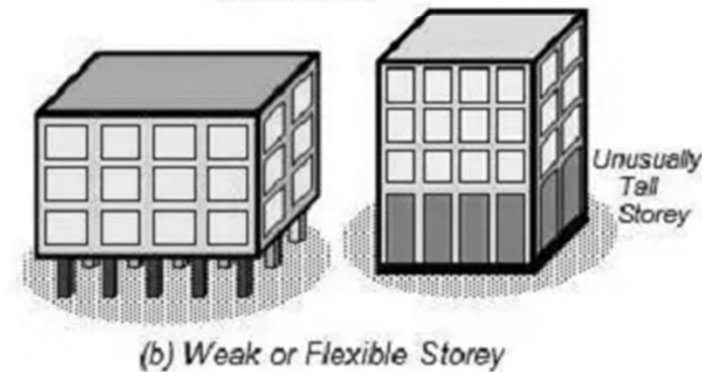
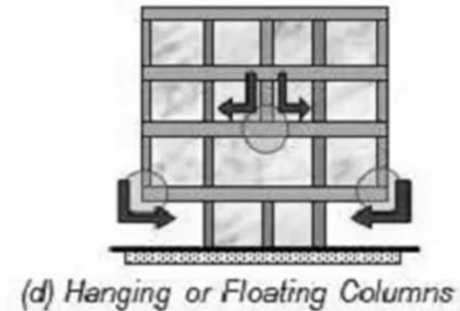
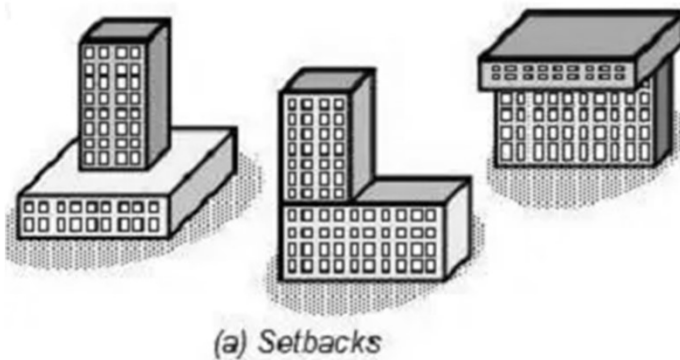


Figure 2: Simple plan shape buildings do well during earthquakes.



# Introduction to Earthquake and Its Effects on Buildings

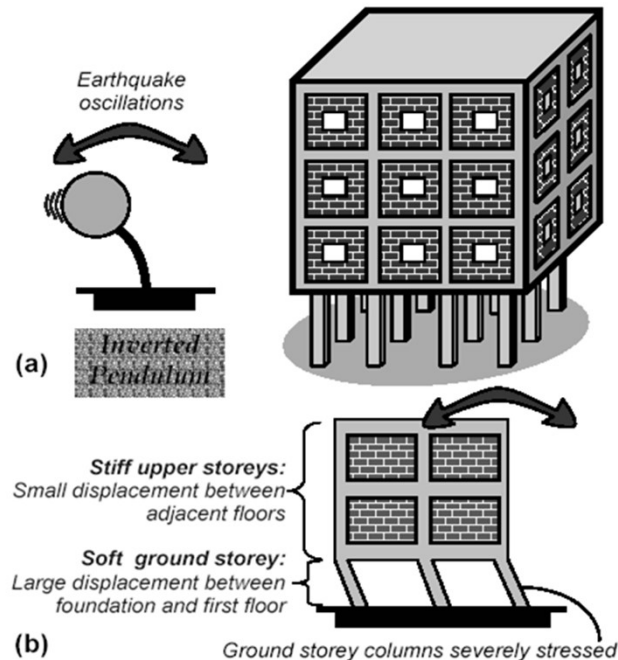
## ❑ Some Undesirable Scenarios





# Introduction to Earthquake and Its Effects on Buildings

## ❑ Some Undesirable Scenarios





# Introduction to Earthquake and Its Effects on Buildings

## □ Earthquake Design Philosophy

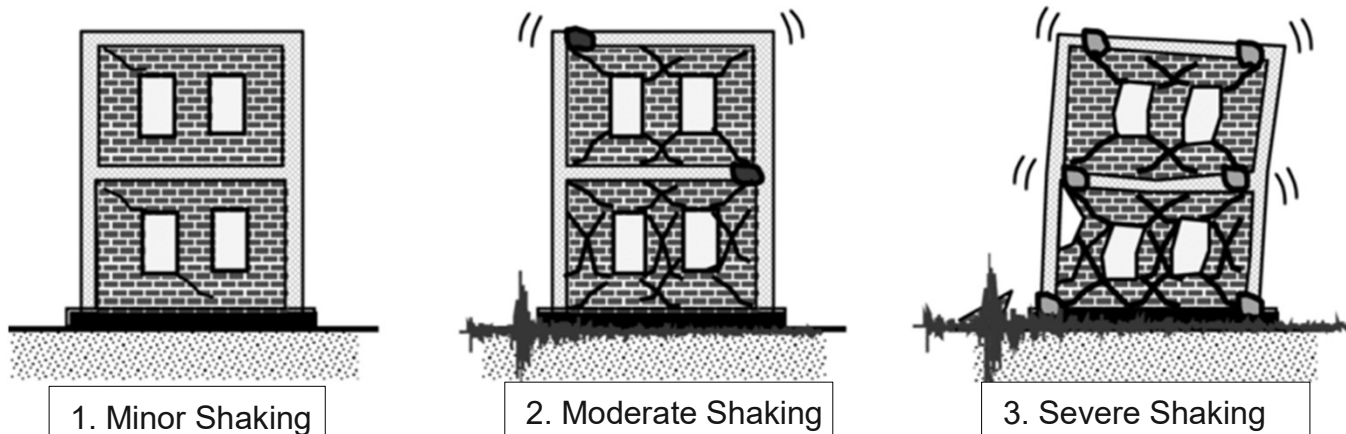
- Designing buildings to respond elastically to earthquakes without suffering any damage might be highly uneconomical.
- Hence, the design philosophy for earthquake resistant design is:  
  
“ To permit controlled damage in order to make the structure economically viable”



# Introduction to Earthquake and Its Effects on Buildings

## □ Earthquake Design Philosophy

- Buildings should be able to resist;
  1. Minor Shaking with No/unnoticeable damage
  2. Moderate Shaking with Minor to moderate structural damage
  3. Severe Shaking with Structural damage, but no collapse

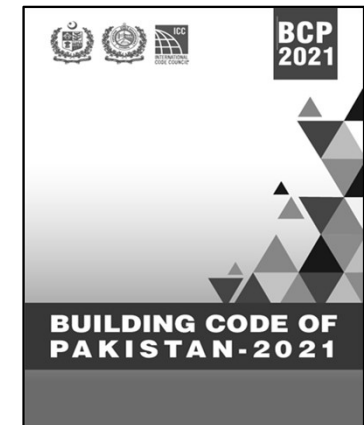
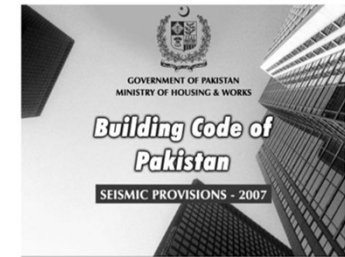




# Seismic Loading Criteria

## ❑ Building Code of Pakistan

- Following the 2005 earthquake in Pakistan, the initial Building Code, BCP SP 2007, was developed, mostly adopting the Uniform Building Code 1997 (UBC 97) except for its seismic maps.
- Recently, this code has undergone a revision to BCP 2021, shifting from its previous alignment with UBC 97 to embracing the International Building Code 2021 (IBC 2021).
- In this course, BCP SP 2007 will be followed.



Brief Visit to BCP 2007





# Seismic Loading Criteria

## □ Seismic Zones

- According to BCP-SP 2007, Pakistan has been divided into five seismic zones. These zones are based on the peak ground acceleration ranges summarized in Table below

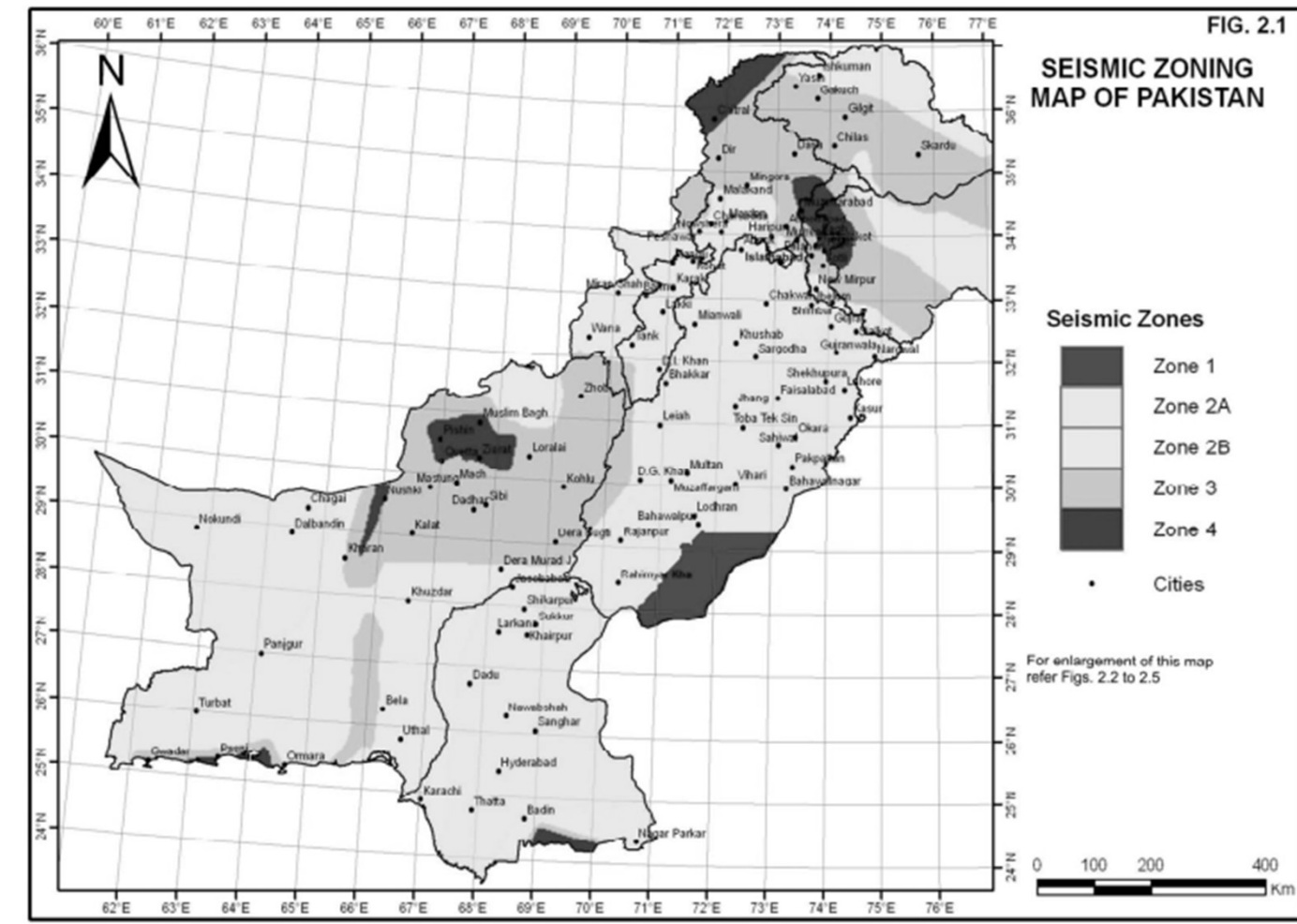
S. No	Seismic Zones	Peak Horizontal Ground Acceleration
1	1	0.05 to 0.08g
2	2A	0.08 to 0.16g
3	2B	0.16 to 0.24g
4	3	0.24 to 0.32g
5	4	>0.32g

Where; g is the acceleration due to gravity



# Seismic Loading Criteria

## □ Seismic Zones





# Seismic Loading Criteria

## ❑ Determination of Lateral force

- The total design seismic force imposed by an earthquake on the structure at its base is referred to as base shear “V” .
- The design seismic force can be determined based on:
  1. Dynamic lateral force procedure (Sec. 5.31, BCP-2007)
  2. Static lateral force procedure (Sec. 5.30.2, BCP-2007)



# Seismic Loading Criteria

## ❑ Determination of Lateral Force

- BCP 5.31 includes information on dynamic lateral force procedures that involve the use of:
  - i. Time history analysis.
  - ii. Response spectrum analysis.
- The details of these methods are presented in sections 5.31.4 and 5.31.6 respectively.



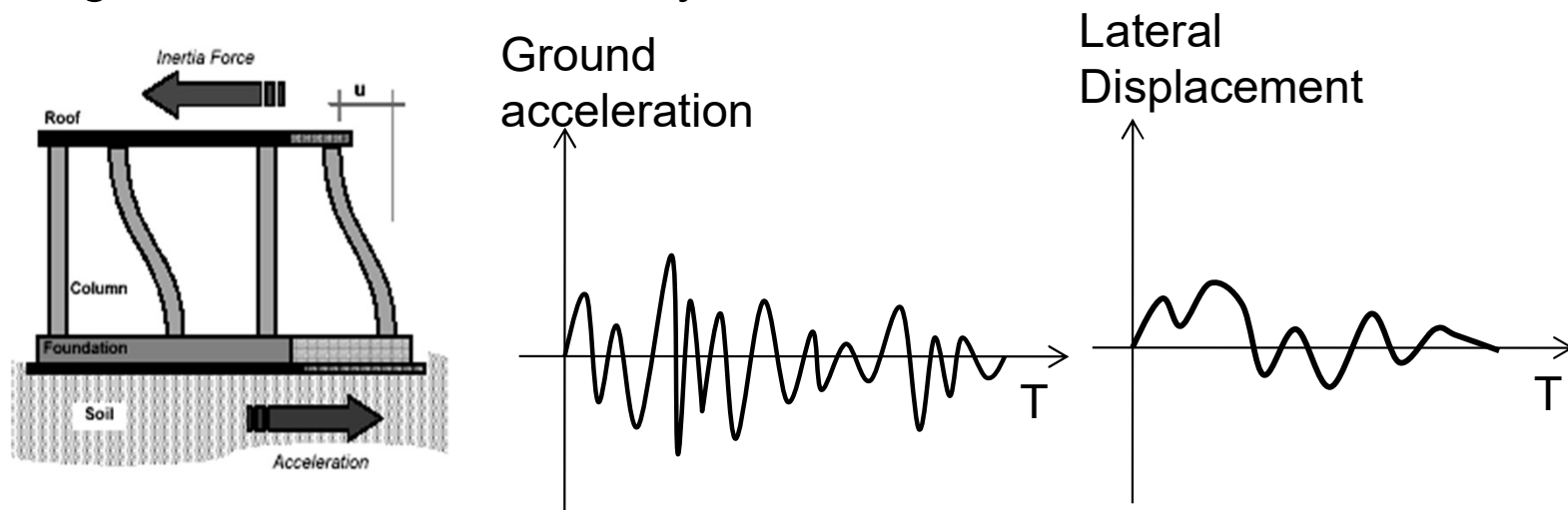
# Seismic Loading Criteria

## □ Determination of Lateral Force

### 1. Dynamic Lateral Force Procedure

#### i. Time History Analysis

- It is the analysis of the dynamic response of a structure at each increment of time when the base is subjected to a specific ground motion time history





# Seismic Loading Criteria

## ❑ Determination of Lateral Force

### 1. Dynamic Lateral Force Procedure

#### ii. Response Spectrum Analysis

- Response Spectrum is a plot of peak response (acceleration, velocity, displacement) of a structure with respect to its natural time period.
- RSA is a linear dynamic statistical method that is used for measuring the maximum seismic response of an elastic structure subjected to ground motion.

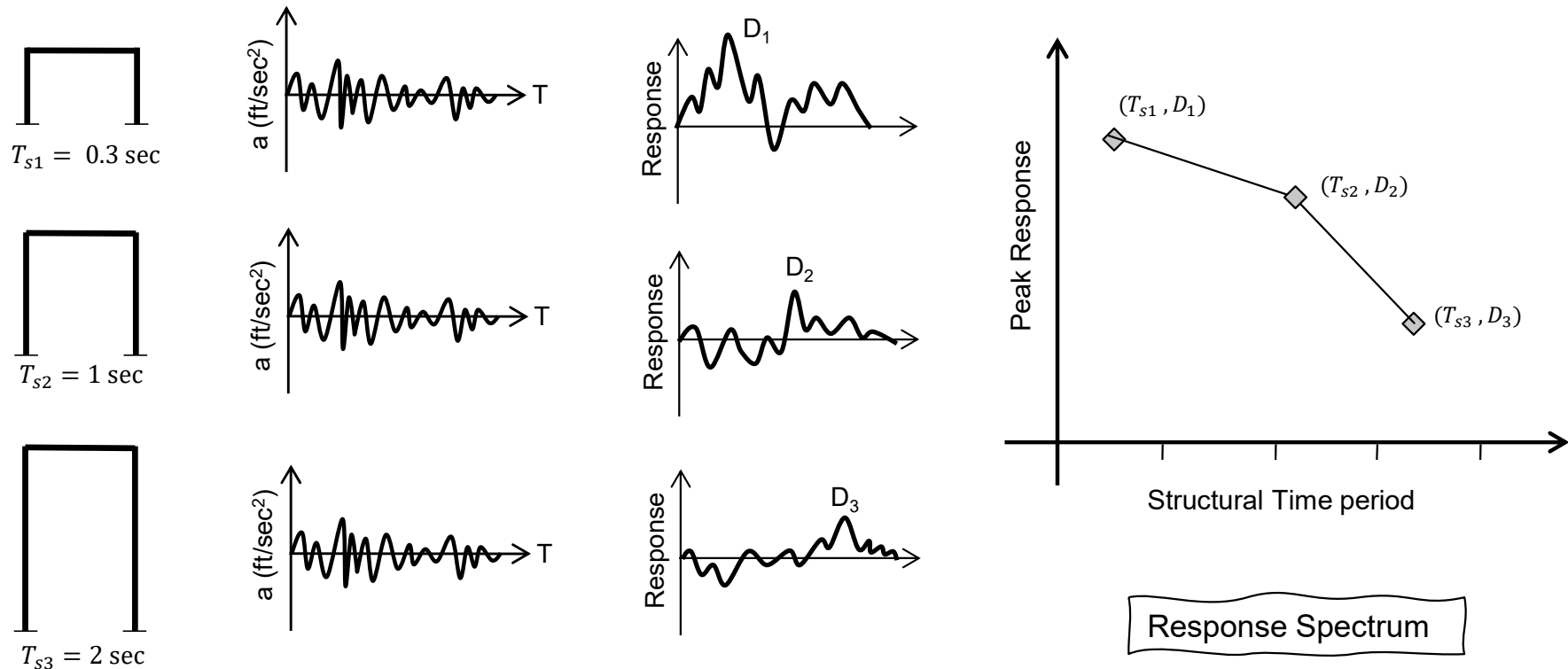


# Seismic Loading Criteria

## □ Determination of Lateral Force

### 1. Dynamic Lateral Force Procedure

#### ii. Response Spectrum Analysis



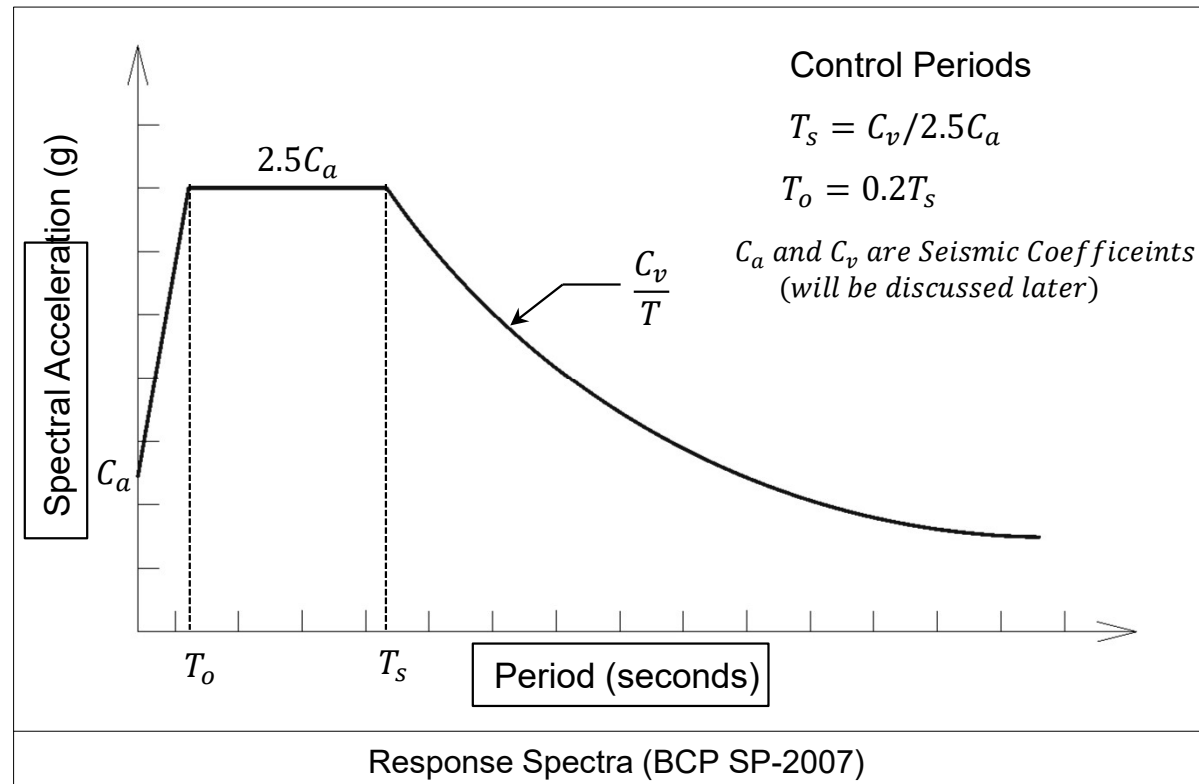


# Seismic Loading Criteria

## □ Determination of Lateral Force

### 1. Dynamic Lateral Force Procedure

#### ii. Response Spectrum Analysis





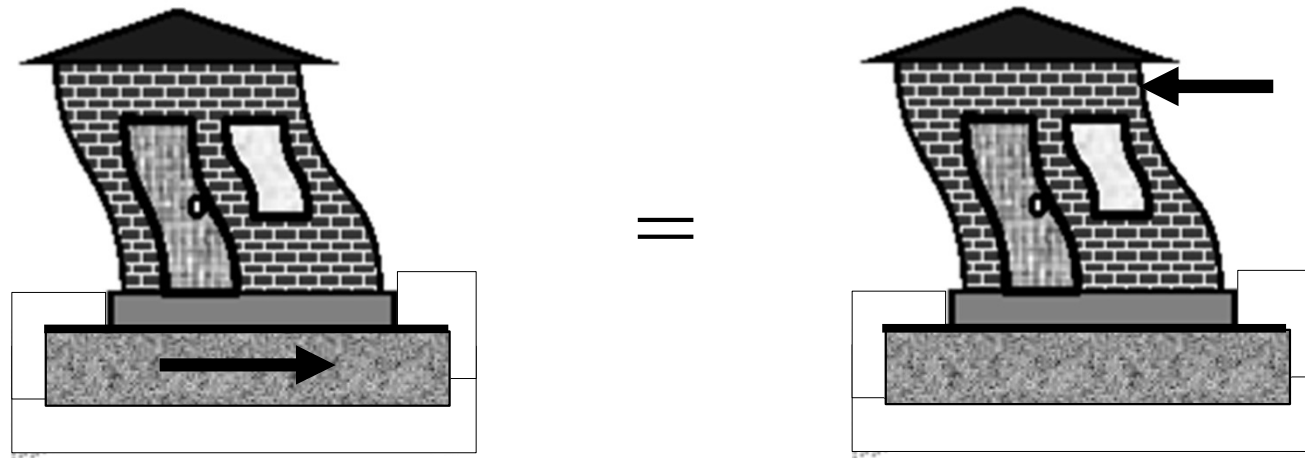


# Seismic Loading Criteria

## □ Determination of Lateral Force

### 2. Static Lateral Force Procedure (5.30.2)

- The equivalent static lateral force method is a simplified technique to transform the effect of dynamic loading of an expected earthquake by a static force.





# Seismic Loading Criteria

## ❑ Determination of Lateral Force

### 2. Static Lateral Force Procedure (5.30.2)

- The total design base shear ( $V$ ) in a given direction can be determined from the following formula;

$$V = \frac{C_v I}{RT} W$$

Where;

$C_v$  = Seismic coefficient (Table 5.16).

$I$  = Seismic importance factor (Table 5.10)

$R$  = Numerical coefficient representative of inherent over strength and global ductility capacity of lateral force-resisting systems (Table 5.13).

$W$  = The total seismic dead load defined in Section 5.30.1.1



# Seismic Loading Criteria

## ❑ Determination of Lateral Force

### 2. Static Lateral Force Procedure (5.30.2)

- Base Shear Limits

$$V_{min} = 0.11C_aIW$$

$$V_{max} = \frac{2.5C_aI}{R}W$$

In addition, for seismic zone 4, the total base shear shall also not be less than ;  $V = (0.8ZN_vI/R)W$

Where;  $N_v$  = near source factor (Table 5.19)

$Z$  = Seismic zone factor (Table 5.9)



# Static Lateral Force Procedure

## ❑ Steps involved in determination of Base Shear

- Step 1: Find Site Specific details
- Step 2: Select Seismic Coefficients
- Step 3: Select Seismic Importance factor
- Step 4: Select Response Modification factor
- Step 5: Determine structure's time period
- Step 6: Calculate base shear and apply base shear limits
- Step 7: Distribute base shear in vertical direction



# Static Lateral Force Procedure

## □ Step 1: Find Site Specific Details

- Following list of data needs to be obtained:
  - i. Seismic Zone
  - ii. Soil Profile Type
  - iii. Past earthquake magnitude (required only for Zone 4).
  - iv. Closest distance to known seismic source (required only for Zone 4).



# Static Lateral Force Procedure

## □ Step 1: Find Site Specific Details

### i. Seismic Zone

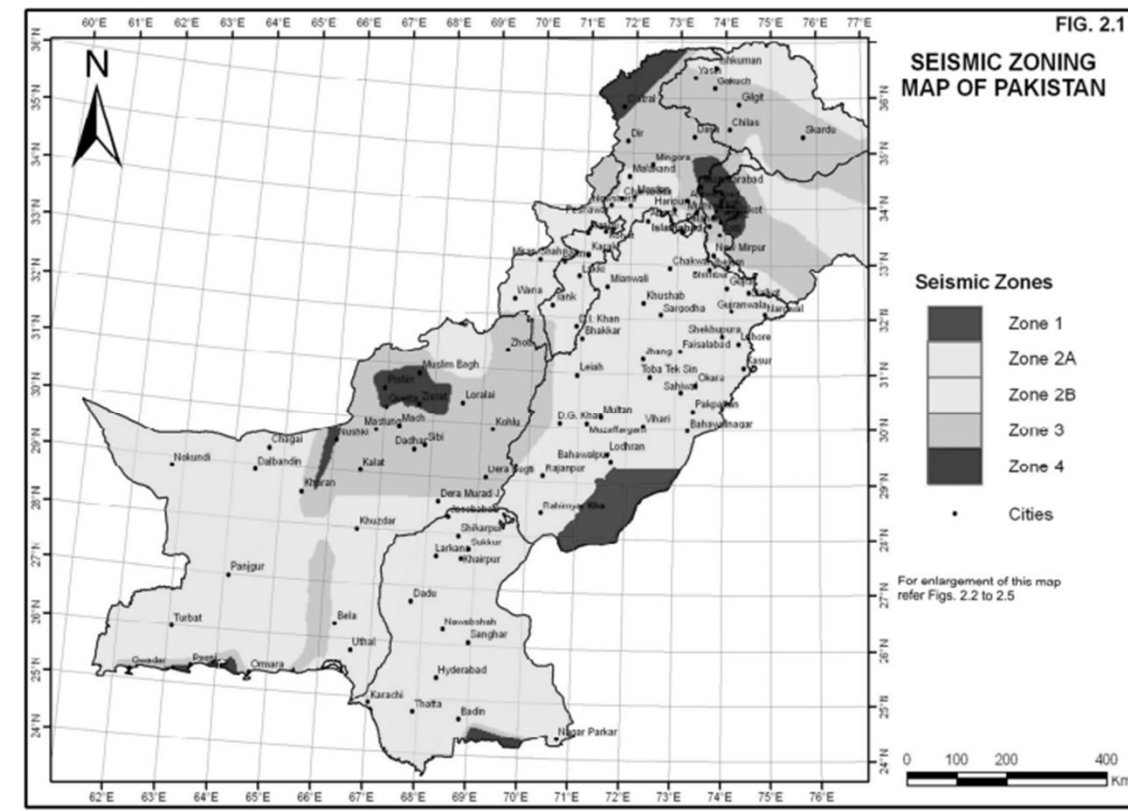
- Seismic Zone for a given site can be selected from the seismic zoning map of the country.
- The seismic zoning map of Pakistan is given in Figure 2.1, while Seismic zoning map of each province have been provided in Figures 2.2 to 2.5 of BCP-SP 2007, are shown in next slides.
- Table 2.2 of BCP lists the seismic zones for all tehsils of the country. This table has been included as an appendix to this Lecture.



# Static Lateral Force Procedure

## □ Step 1: Find Site Specific Details

### i. Seismic Zone





# Static Lateral Force Procedure

## □ Step 1: Find Site Specific Details

### i. Seismic Zone

- After selecting the Seismic zone, Seismic zone factor  $Z$  can be chosen from 5.9 of BCP SP 2007.

Table 5.9 —Seismic Zone Factor $Z$					
Zone	1	2A	2B	3	4
$Z$	0.075	0.15	0.20	0.30	0.40





# Static Lateral Force Procedure

## □ Step 1: Find Site Specific Details

### ii. Soil Profile Type

- Soil Profile Types have been defined in Table 4.1 of BCP SP 2007.

Table 4.1 — Soil Profile Types				
Soil Profile Type	Generic Description	Average Soil Properties for Top 100ft of Soil Profile		
		Shear wave velocity (ft/s)	SPT N(blow/ft)	Undrained Shear Strength (psf)
$S_A$	Hard rock	> 4920	---	---
$S_B$	Rock	2460 to 4920		
$S_C$	Very dense Soil & soft Rock	1150 to 2460	> 50	> 2000
$S_D$	Stiff soil Profile	575 to 1150	15 to 50	1000 to 2000
$S_E$	Soft soil	< 575	< 15	< 1000
$S_F$	Soil requiring site-specific Evaluation			



# Static Lateral Force Procedure

## □ Step 1: Find Site Specific Details

### ii. Past Earthquake Magnitude

- This is required only for seismic zone 4 to decide about seismic source type.

Table 5.20 —Seismic Source Type			
Seismic Source Type	Seismic Source Description	Maximum moment Magnitude	Slip Rate (mm/yr)
A	Faults that can produce large magnitude events and that have high rate of seismic activity	$M \geq 7.0$	$SR \geq 5$
B	All faults other than Types A and C	$M \geq 7.0$ $M < 7.0$ $M \geq 6.5$	$SR < 5$ $SR > 5$ $SR < 5$
C	Faults that are not capable of producing large magnitude events and that have relatively low rate of seismic activity	$M < 6.5$	$SR \leq 2$



# Static Lateral Force Procedure

## ❑ Step 1: Find Site Specific Details

### ii. Distance to Known Seismic Source

- Distance to known seismic source is also required to determine additional coefficients for zone 4.



# Static Lateral Force Procedure

## □ Step 2: Determination of Seismic Coefficients

- The values of seismic acceleration coefficient  $C_a$  can be taken from Table 5.16 of BCP SP 2007.

Table 5.16 —Seismic Coefficients $C_a$					
Soil Profile Type	Seismic Zone Factor, $Z$				
	$Z = 0.075$	$Z = 0.15$	$Z = 0.20$	$Z = 0.3$	$Z = 0.4$
$S_A$	0.06	0.12	0.16	0.24	$0.32N_a$
$S_B$	0.08	0.15	0.20	0.30	$0.40N_a$
$S_C$	0.09	0.18	0.24	0.33	$0.40N_a$
$S_D$	0.12	0.22	0.28	0.36	$0.44N_a$
$S_E$	0.19	0.30	0.34	0.36	$0.36N_a$
$S_F$	See Footnote 1				

[1] Site-specific geotechnical investigation and dynamic site response analysis shall be performed to determine seismic coefficients for Soil Profile Type  $S_F$ .



# Static Lateral Force Procedure

## □ Step 2: Determination of Seismic Coefficients

- The values of seismic velocity coefficient  $C_v$  can be taken from Table 5.17 of BCP SP 2007.

Table 5.17 —Seismic Coefficients $C_v$					
Soil Profile Type	Seismic Zone Factor, $Z$				
	$Z = 0.075$	$Z = 0.15$	$Z = 0.20$	$Z = 0.3$	$Z = 0.4$
$S_A$	0.06	0.12	0.16	0.24	$0.32N_v$
$S_B$	0.08	0.15	0.20	0.30	$0.40N_v$
$S_C$	0.13	0.25	0.32	0.45	$0.56N_v$
$S_D$	0.18	0.32	0.40	0.54	$0.64N_v$
$S_E$	0.26	0.50	0.64	0.84	$0.96N_v$
$S_F$	See Footnote 1				

[1] Site-specific geotechnical investigation and dynamic site response analysis shall be performed to determine seismic coefficients for Soil Profile Type  $S_F$ .



# Static Lateral Force Procedure

## □ Step 2: Determination of Seismic Coefficients

- The values  $N_a$  and  $N_v$  can be taken from Tables 5.18 & 5.19 of BCP SP 2007. These values are required for Zone 4 only.

Table 5.18 —Near Source Factor, $N_a$			
Seismic Source Type	Closest Distance To Known Seismic Source		
	$\leq 2$ km	5 km	$\geq 10$ km
A	1.5	1.2	1.0
B	1.3	1.0	1.0
C	1.0	1.0	1.0



# Static Lateral Force Procedure

## □ Step 2: Determination of Seismic Coefficients

- The values  $N_a$  and  $N_v$  can be taken from Tables 5.18 & 5.19 of BCP SP 2007. These values are required for Zone 4 only.

Table 5.19 —Near Source Factor, $N_v$				
Seismic Source Type	Closest Distance To Known Seismic Source			
	$\leq 2$ km	5 km	10 km	$\geq 15$ km
A	2	1.6	1.2	1.0
B	1.6	1.2	1.0	1.0
C	1.0	1.0	1.0	1.0



# Static Lateral Force Procedure

## ❑ Step 3: Determination of Seismic Importance Factor

- Based on the occupancy category, Seismic Importance Factor “I” can be selected from Table 5.10 of BCP SP 2007.





# Static Lateral Force Procedure

## □ Step 3: Determination of Seismic Importance Factor

Table 5.10 — Occupancy Category		
Occupancy Category	Occupancy or Function of Structure	Seismic Importance factor, $I$
Essential facilities	<ul style="list-style-type: none"> <li>Group I, Division 1 Occupancies having surgery and emergency treatment areas</li> <li>Fire and police stations</li> <li>Garages and shelters for emergency vehicles and emergency aircraft</li> <li>Structures and shelters in emergency-preparedness centers</li> <li>Aviation control towers</li> <li>Structures and equipment in government communication centers and other facilities required for emergency response</li> <li>Standby power-generating equipment for Category 1 facilities</li> <li>Tanks or other structures containing housing or supporting water or other fire-suppression material or equipment required for the protection of Category 1, 2 or 3 structures</li> </ul>	1.25
Hazardous facilities	<ul style="list-style-type: none"> <li>Group H, Divisions 1, 2, 6 and 7 Occupancies and structures therein housing or supporting toxic or explosive chemicals or substances</li> <li>Nonbuilding structures housing, supporting or containing quantities of toxic or explosive substances that, if contained within a building, would cause that building to be classified as a Group H, Division 1, 2 or 7 Occupancy</li> </ul>	1.25



# Static Lateral Force Procedure

## ❑ Step 3: Determination of Seismic Importance Factor

Table 5.10 — Occupancy Category		
Occupancy Category	Occupancy or Function of Structure	Seismic Importance factor, $I$
Special Occupancy Category	<ul style="list-style-type: none"> <li>Group A, Divisions 1, 2 and 2.1 Occupancies</li> <li>Buildings housing Group E, Divisions 1 and 3 Occupancies with a capacity greater than 300 students</li> <li>Buildings housing Group B Occupancies used for college or adult education with a capacity greater than 500 students</li> <li>Group I, Divisions 1 and 2 Occupancies with 50 or more resident incapacitated patients, but not included in Category 1</li> <li>Group I, Division 3 Occupancies</li> <li>All structures with an occupancy greater than 5,000 persons</li> <li>Structures and equipment in power-generating stations, and other public utility facilities not included in Category 1 or Category 2 above, and required for continued operation</li> </ul>	1.00
Standard occupancy structures	<ul style="list-style-type: none"> <li>All structures housing occupancies or having functions not listed in Category 1, 2 or 3 and Group U Occupancy towers</li> </ul>	1.00
Miscellaneous structures	Group U Occupancies except for towers	1.00



# Static Lateral Force Procedure

## ❑ Step 4: Determination of Response Modification Factor

- Response modification factor basically reduces base shear “ $V$ ” to make the system economical.
- However, the structure will suffer some damage as explained in the earthquake design philosophy.
- $R$  depends on overall structural response of the structure under lateral loading. For structures exhibiting good performance,  $R$  will be high.



# Static Lateral Force Procedure

## ❑ Step 4: Determination of Response Modification Factor

Table 5.13 – Structural Systems		
Basic Structural System	Lateral Force resisting System Description	<i>R</i>
1. Bearing wall system	<b>1. Light-framed walls with shear panels</b>	
	a. Wood structural panel walls for structures three stories or less	5.5
	b. All other light-framed walls	4.5
	<b>2. Shear walls</b>	
	a. Concrete	4.5
	b. Masonry	4.5
	<b>3. Light steel-framed bearing walls with tension-only bracing</b>	2.8
	<b>4. Braced frames where bracing carries gravity load</b>	
	a. Steel	4.4
	b. Concrete	2.8
	c. Heavy timber	2.8



# Static Lateral Force Procedure

## □ Step 4: Determination of Response Modification Factor

Table 5.13 – Structural Systems		
Basic Structural System	Lateral Force resisting System Description	<i>R</i>
2. Building frame system	1. Steel eccentrically braced frame (EBF)	7
	2. Light-framed walls with shear panels	
	a. Wood structural panel walls for structures three stories or less	6.5
	b. b. All other light-framed walls	5
	3. Shear walls	
	a. Concrete	5.5
	b. Masonry	5.5
	4. Ordinary braced frames	
	a. Steel	5.6
	b. Concrete	5.6
	c. Heavy timber	5.6
	5. Special concentrically braced frames	6.4



# Static Lateral Force Procedure

## ❑ Step 4: Determination of Response Modification Factor

Table 5.13 – Structural Systems		
Basic Structural System	Lateral Force resisting System Description	<i>R</i>
3. Moment-resisting frame system	1. Special moment-resisting frame (SMRF)	
	a. Steel	8.5
	b. Concrete	8.5
	2. Masonry moment-resisting wall frame (MMRWF)	6.5
	3. Concrete intermediate moment-resisting frame (IMRF)	5.5
	4. Ordinary moment-resisting frame (OMRF)	
	a. Steel	4.5
	b. Concrete	3.5
	5. Special truss moment frames of steel (STMF)	6.5

The Table additionally provides the R-Factor values for other structural systems such as Dual Systems, Cantilevered Column Building Systems, Shear Wall-Frame Interaction Systems, and Undefined Systems.



# Static Lateral Force Procedure

## □ Step 5: Determination of Response Modification Factor

### ❖ Structural Period (5.30.2.2)

- For all buildings, the value  $T$  may be approximated from the following formula:

$$T = C_t(h_n)^{3/4}$$

Where;

$C_t = 0.035$  for steel moment-resisting frames.

$C_t = 0.030$  for reinforced concrete moment-resisting frames and eccentrically braced frames.  $C_t = 0.020$  for all other buildings.

$h_n$  = Actual height (feet or meters) of the building above the base to the  $n$ th level.



# Static Lateral Force Procedure

## □ Step 6: Calculation of Base shear and applying Base Shear limits

$$V = \frac{C_v I}{RT} W$$

The calculated value of “V” should be within the following limits

$$V_{min} = 0.11C_a I W$$

$$V_{max} = \frac{2.5C_a I}{R} W$$

In addition, for seismic zone 4, the total base shear shall also not be less than ;  $V = (0.8ZN_v I / R) W$





# Static Lateral Force Procedure

## □ Step 7: Vertical Distribution of Base Shear

- As per section 5.30.5 of BCP SP 2007, The lateral force at a particular story level  $x$  of the structure is given as:

$$F_x = (V - F_t) \frac{w_x h_x}{\sum_{i=1}^{i=n} (W_i h_i)}$$

Where;

- $n = \text{Number of stories}$
- $F_t = 0.07TV \leq 0.25V$  (may be considered as zero where  $T \leq 0.7\text{sec}$ )

$F_t$  is an additional concentrated force that is applied to the top level (i.e., the roof) in addition to the  $F_x$  force at that level.



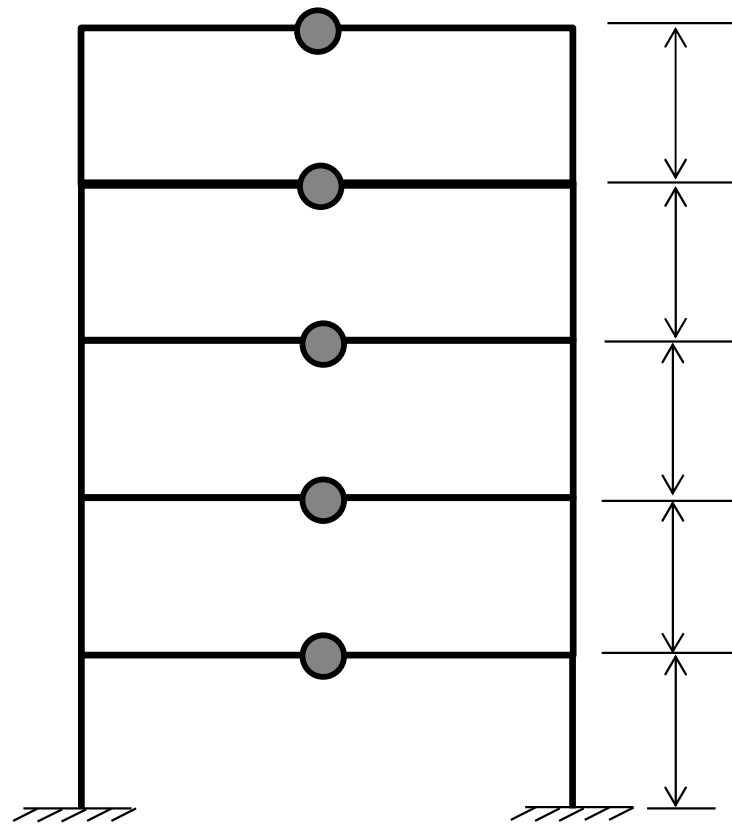
## Example 5.1

### □ Problem Statement

- A five-story reinforced concrete residential building as shown in figure on the next slide, is required to be designed as Special Moment-Resisting Frame (SMRF) System. The structure is situated in Abbottabad, KP. It has been found from the geotechnical investigation that the soil at the location is class  $S_D$ .
  - a. Calculate*** the base shear using static later force procedure
  - b. Determine*** joint forces at each story level



## Example 5.1





## Example 5.1

- **Solution**

- **Given Data**

Structural system: SMRF (concrete)

Occupancy : Residential

Site : Abbottabad, KP

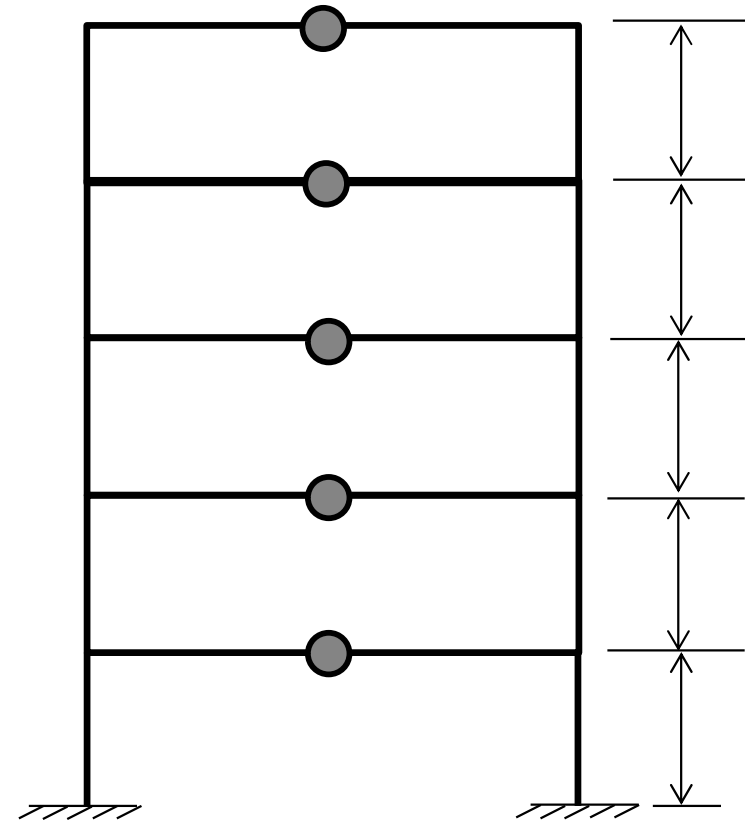
Soil Profile Type:  $S_D$

Number of stories: 5

Story height: 12ft each

- **Required Data**

- a. Base Shear  $V$
- b. Story Forces,  $F_x$



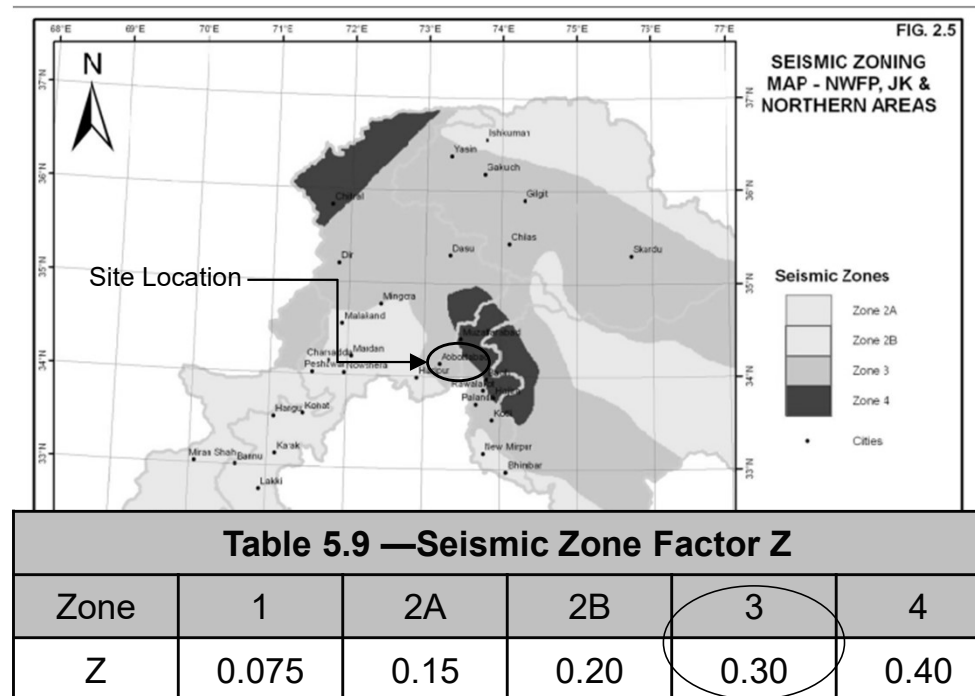


## Example 5.1

- Solution**

- **Step 1: Site Specific Details**

- Abbottabad falls in Zone 3 (From Seismic Zoning Map).
- Seismic Zone factor “Z” for zone 3 is 0.3 (From Table 5.9)





## Example 5.1

### • Solution

#### ➤ Step 2: Selection of Seismic Coefficients ( $C_a$ and $C_v$ )

- Knowing Soil profile Type and Seismic zone factor, the values of  $C_a$  and  $C_v$  can be picked up from the relevant tables.

Table 5.16 —Seismic Coefficients $C_a$					
Soil Profile Type	Seismic Zone Factor, Z				
	Z = 0.075	Z = 0.15	Z = 0.20	Z = 0.3	Z = 0.4
$S_A$	0.06	0.12	0.16	0.24	$0.32N_a$
$S_B$	0.08	0.15	0.20	0.30	$0.40N_a$
$S_C$	0.09	0.18	0.24	0.33	$0.40N_a$
$S_D$	0.12	0.22	0.28	0.36	$0.44N_a$
$S_E$	0.19	0.30	0.34	0.36	$0.36N_a$
$S_F$	See Footnote 1				



## Example 5.1

### • Solution

- **Step 2: Selection of Seismic Coefficients ( $C_a$  and  $C_v$ )**
- Knowing Soil profile Type and Seismic zone factor, the values of  $C_a$  and  $C_v$  can be picked up from the relevant tables.

Table 5.17 —Seismic Coefficients $C_v$					
Soil Profile Type	Seismic Zone Factor, Z				
	Z = 0.075	Z = 0.15	Z = 0.20	Z = 0.3	Z = 0.4
$S_A$	0.06	0.12	0.16	0.24	$0.32N_v$
$S_B$	0.08	0.15	0.20	0.30	$0.40N_v$
$S_C$	0.13	0.25	0.32	0.45	$0.56N_v$
$S_D$	0.18	0.32	0.40	0.54	$0.64N_v$
$S_E$	0.26	0.50	0.64	0.84	$0.96N_v$
$S_F$	See Footnote 1				



## Example 5.1

### • Solution

#### ➤ Step 3: Selection of Seismic Importance Factor (I)

- The occupancy of the given building is residential which comes under *Standard Occupancy structures* category.

Table 5.10 — Occupancy Category		
Occupancy Category	Occupancy or Function of Structure	I
Special Occupancy Category	Group A, Divisions 1, 2 and 2.1 Occupancies Buildings housing Group E, Divisions 1 and 3 Occupancies with a capacity greater than 300 students Buildings housing Group B Occupancies used for college or adult education with a capacity greater than 500 students Group I, Divisions 1 and 2 Occupancies with 50 or more resident incapacitated patients, but not included in Category 1 Group I, Division 3 Occupancies All structures with an occupancy greater than 5,000 persons Structures and equipment in power-generating stations, and other public utility facilities not included in Category 1 or Category 2 above, and required for continued operation	1.00
Standard occupancy structures	All structures housing occupancies or having functions not listed in Category 1, 2 or 3 and Group U Occupancy towers	1.00
Miscellaneous structures	Group U Occupancies except for towers	1.00





## Example 5.1

- Solution**

- **Step 4: Selection of Response Modification Factor (R)**

- For SMRF systems composed of concrete, the value of R from Table 5.13 is 8.5.

Table 5.13 – Structural Systems		
Basic Structural System	Lateral Force resisting System Description	R
3. Moment-resisting frame system	1.Special moment-resisting frame (SMRF)	
	a. Steel	8.5
	b. Concrete	8.5
	2.Masonry moment-resisting wall frame (MMRWF)	6.5
	3.Concrete intermediate moment-resisting frame (IMRF)	5.5
	4.Ordinary moment-resisting frame (OMRF)	
	a. Steel	4.5
	b. Concrete	3.5
	5.Special truss moment frames of steel (STMF)	6.5



## Example 5.1

- **Solution**

- **Step 5: Determine Structure's Time period**

- The value of  $T$  can be determined as follows;

$$T = C_t(h_n)^{3/4}$$

Here;

$C_t = 0.030$  for concrete frames

$h = \text{Total height of structure} = 5 \times 12 = 60'$

By substituting relevant values, we get

$$T = 0.030(60)^{3/4} = 0.647 \text{ sec}$$



## Example 5.1

- **Solution**

- **Step 6: Calculate Base shear and apply base shear limits**

$$V = \frac{C_v I}{RT} W$$

Here;

$$\begin{aligned} W &= W_1 + W_2 + W_3 + W_4 + W_5 = 800 + 800 + 800 + 800 + 700 \\ &= 3900 \text{ kips} \end{aligned}$$

Substituting values and solving gives;

$$V = \frac{0.54 \times 1}{8.5 \times 0.647} \times (3900) = 382.94 \text{ kips}$$



## Example 5.1

- **Solution**

- **Step 6: Calculate Base shear and apply base shear limits**

Applying base shear limits;

$$V_{min} = 0.11C_aIW = 0.11(0.36)(1)(3900) = 154.44kips$$

And;

$$V_{max} = \frac{2.5C_aI}{R}W = \frac{2.5 \times 0.36 \times 1 \times 3900}{8.5} = 412.94kips$$

$$V_{min} < V < V_{max} \rightarrow OK!$$

Therefore;

$$V = 382.94kips$$

[ This is the required answer for part (a) ]



## Example 5.1

- **Solution**

- **Step 7: Vertical Distribution of Base shear/ Story forces**

The joint force  $F_x$  at level  $x$  is given by

$$F_x = (V - F_t) \frac{w_x h_x}{\sum_{i=1}^{i=n} (W_i h_i)}$$

Since value of  $T = 0.647 < 0.7$ , therefore the value of  $F_t = 0$

Using the above equation, the story forces have been calculated and tabulated on the next slide.



## Example 5.1

- Solution**

➤ **Step 7: Vertical Distribution of Base shear/ Story forces**

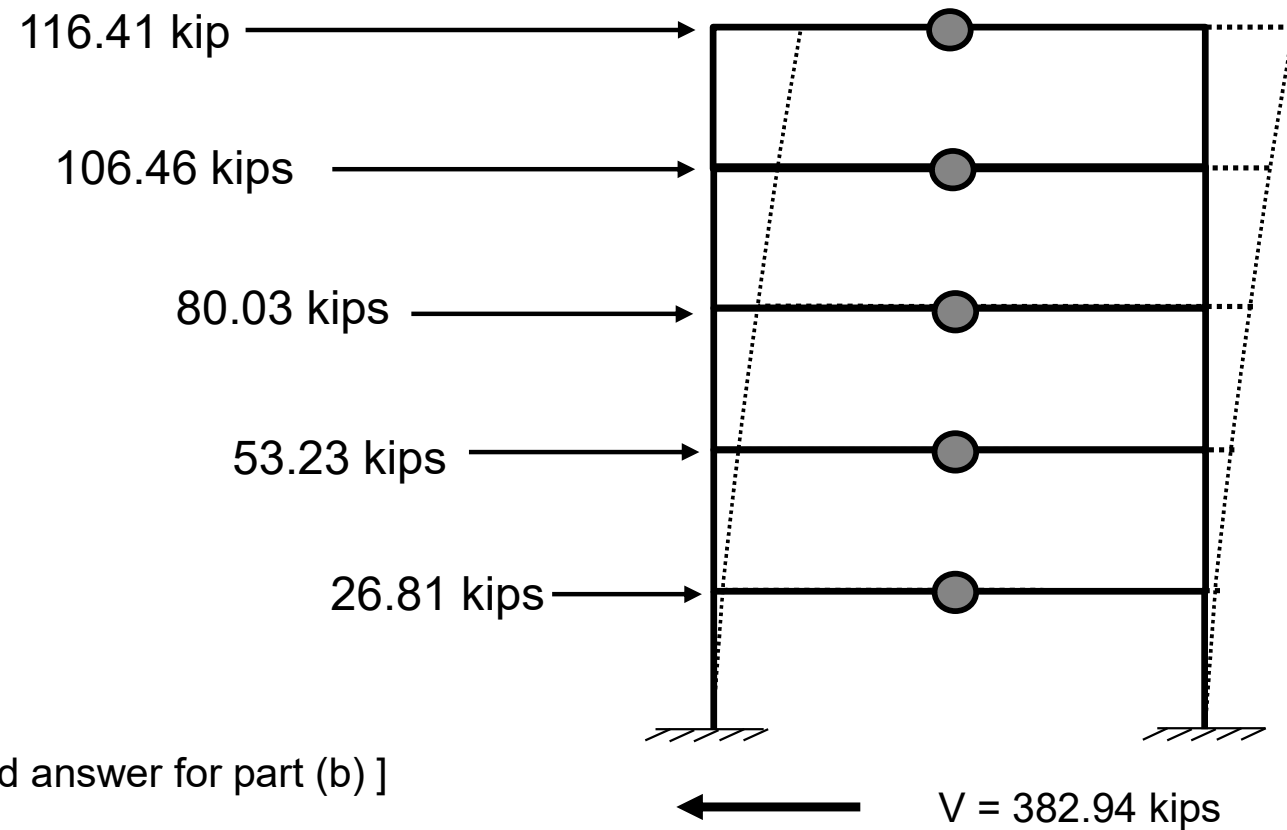
Vertical Distribution of Base Shear						
<i>Levels</i>	$h_x$ (ft)	$W_x$ (kips)	$W_x h_x$	$\frac{W_x h_x}{\sum W_i h_i}$	$V$ (kips)	$F_x = V \frac{W_x h_x}{\sum W_i h_i} (kips)$
1	12	800.00	9600	0.070	382.94	26.81
2	24	800.00	19200	0.139	382.94	53.23
3	36	800.00	28800	0.209	382.94	80.03
4	48	800.00	38400	0.278	382.94	106.46
5	60	700.00	42000	0.304	382.94	116.41
$\sum W_i h_i =$			<b>138000</b>	Check: $\sum F_x = V$ ; $382.94 = 382.94 \rightarrow OK!$		



## Example 5.1

- Solution**

- **Step 7: Vertical Distribution of Base shear/ Story forces**

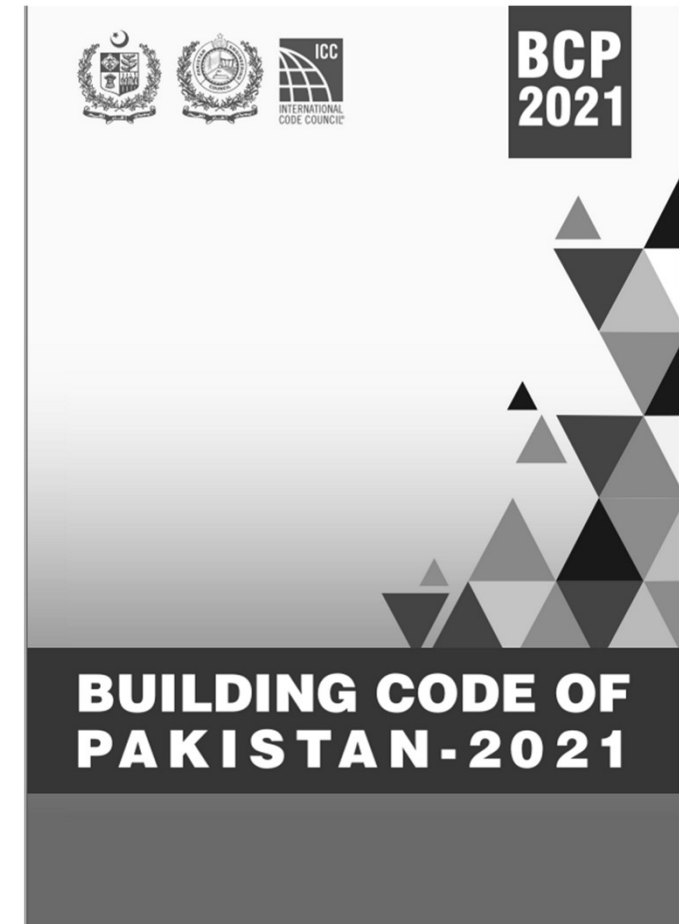




# Introduction to BCP 2021

## □ BCP 2021

- The Building Code of Pakistan 2021 (BCP 2021), issued in 2022, adopts the International Building Code (IBC 2021) except for seismic maps.
- BCP 2021 integrates results from a recently updated probabilistic seismic hazard analysis (PSHA) carried out with cutting-edge methodologies and the latest data sources available.







# Introduction to BCP 2021

## □ Seismic Loading Criteria in BCP 2021

- In BCP 2021, the seismic analysis is based on the Maximum Considered Earthquake (MCE), which is a very rare type of earthquake with a 2% probability of exceedance in 50 years (return period of 2475 years).
- In contrast to BCP-SP 2007, which relied on a singular Peak Ground Acceleration (PGA) across all time period values, BCP-2021 introduces two distinct spectral accelerations at specified time periods.
  1. Short Period Spectral Acceleration ( $S_s$ )  $\rightarrow T = 0.2$  sec
  2. Long Period spectral Acceleration ( $S_1$ )  $\rightarrow T = 1.0$  sec



# Introduction to BCP 2021

## □ Seismic Loading Criteria in BCP 2021

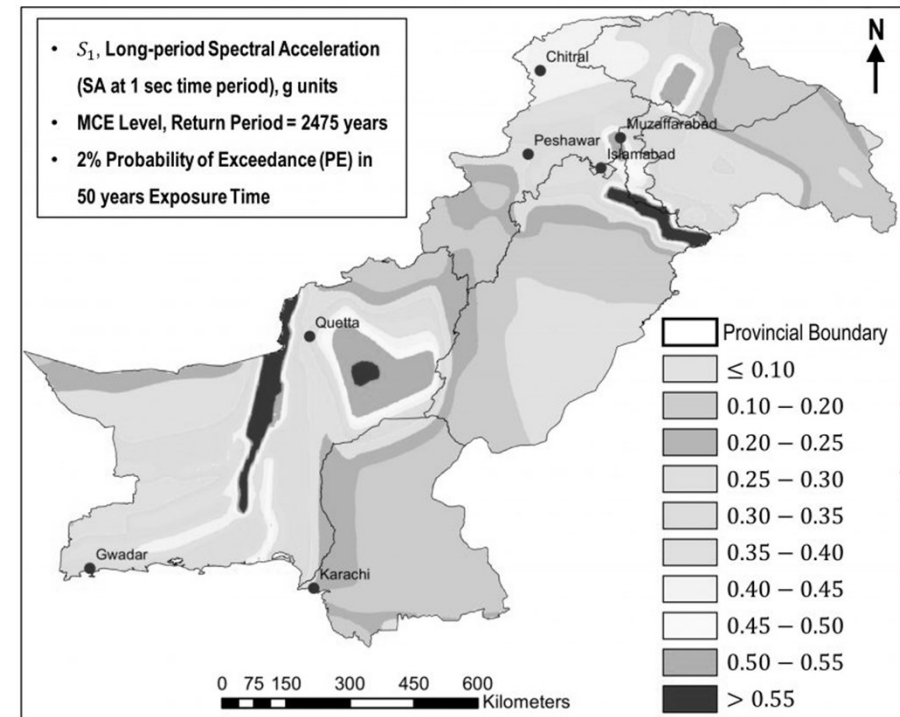
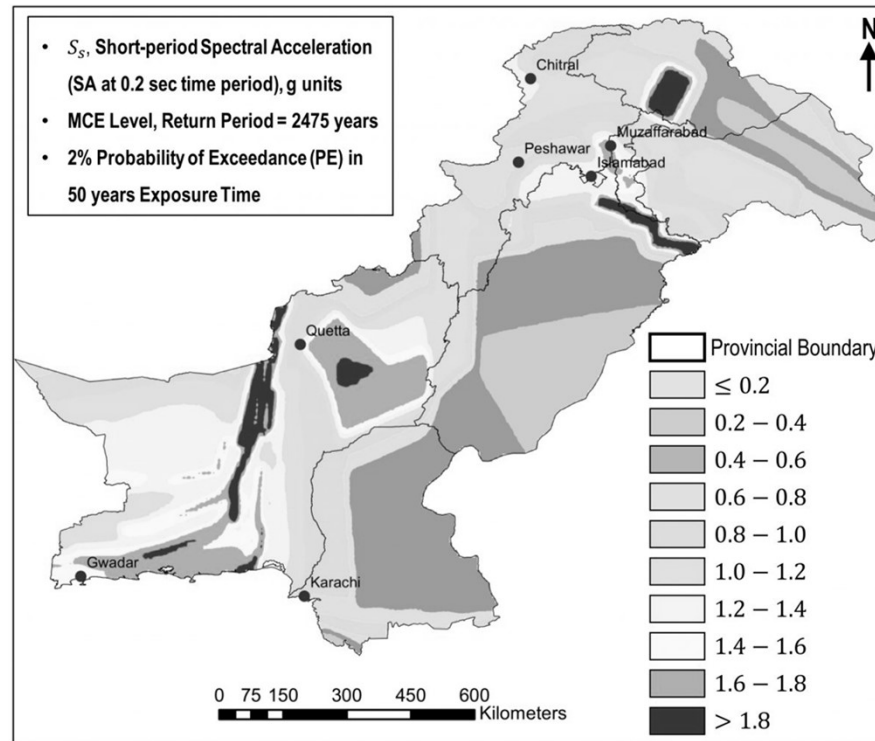
- The parameters  $S_s$  and  $S_1$  values are selected from the relevant Seismic hazard map of country.
- For Pakistan, these values are provided in figures 1613.2.1(1) and 1613.2.1(2) of BCP-2021 as shown next.



# Introduction to BCP 2021

## □ Seismic Loading Criteria in BCP 2021

### ❖ Seismic Map of Pakistan





# Introduction to BCP 2021

## □ Seismic Loading Criteria in BCP 2021

### ❖ Adjusted MCE Level Spectral Acceleration Parameters

- It should be noted that the  $S_s$  and  $S_1$  correspond to MCE level, excluding local site conditions.
- To account for the local site effects, the parameters shall be adjusted as follows;

$$S_{MS} = F_a \times S_s$$

$$S_{M1} = F_v \times S_1$$

Where;

$F_a$  and  $F_v$  which are site coefficients provided in Tables 1613.2.2(1) and 1613.2.2(2) of BCP-2021 (shown next).



# Introduction to BCP 2021

## □ Seismic Loading Criteria in BCP 2021

### ❖ Adjusted MCE Level Spectral Acceleration Parameters

#### • Site Coefficients $F_a$ and $F_v$

**TABLE 1613.2.3(1)**  
**VALUES OF SITE COEFFICIENT  $F_a^a$**

SITE CLASS	MAPPED RISK TARGETED MAXIMUM CONSIDERED EARTHQUAKE (MCE <sub>R</sub> ) SPECTRAL RESPONSE ACCELERATION PARAMETER AT SHORT PERIOD					
	$S_s \leq 0.25$	$S_s = 0.50$	$S_s = 0.75$	$S_s = 1.00$	$S_s = 1.25$	$S_s \geq 1.5$
A	0.8	0.8	0.8	0.8	0.8	0.8
B	0.9	0.9	0.9	0.9	0.9	0.9
C	1.3	1.3	1.2	1.2	1.2	1.2
D	1.6	1.4	1.2	1.1	1.0	1.0
E	2.4	1.7	1.3	Note b	Note b	Note b
F	Note b	Note b	Note b	Note b	Note b	Note b



# Introduction to BCP 2021

## □ Seismic Loading Criteria in BCP 2021

### ❖ Adjusted MCE Level Spectral Acceleration Parameters

#### • Site Coefficients $F_a$ and $F_v$

**TABLE 1613.2.3(2)**  
**VALUES OF SITE COEFFICIENT  $F_v^a$**

SITE CLASS	MAPPED RISK TARGETED MAXIMUM CONSIDERED EARTHQUAKE ( $MCE_R$ ) SPECTRAL RESPONSE ACCELERATION PARAMETER AT 1-SECOND PERIOD					
	$S_1 \leq 0.1$	$S_1 = 0.2$	$S_1 = 0.3$	$S_1 = 0.4$	$S_1 = 0.5$	$S_1 \geq 0.6$
A	0.8	0.8	0.8	0.8	0.8	0.8
B	0.8	0.8	0.8	0.8	0.8	0.8
C	1.5	1.5	1.5	1.5	1.5	1.4
D	2.4	2.2 <sup>c</sup>	2.0 <sup>c</sup>	1.9 <sup>c</sup>	1.8 <sup>c</sup>	1.7 <sup>c</sup>
E	4.2	3.3 <sup>c</sup>	2.8 <sup>c</sup>	2.4 <sup>c</sup>	2.2 <sup>c</sup>	2.0 <sup>c</sup>
F	Note b	Note b	Note b	Note b	Note b	Note b



# Introduction to BCP 2021

## ❑ Seismic Loading Criteria in BCP 2021

### ❖ Design Response Spectral Acceleration Parameters

- The five-percent damped design response spectral acceleration at short-period  $S_{DS}$  and at long period  $S_{D1}$  shall be determined from the following equations:

$$S_{DS} = \frac{2}{3} S_{MS} \quad \text{and} \quad S_{D1} = \frac{2}{3} S_{M1}$$

Hence,

$$S_{DS} = \frac{2}{3} S_S \times F_a$$

$$S_{D1} = \frac{2}{3} S_1 \times F_v$$



# Introduction to BCP 2021

## ❑ Seismic Loading Criteria in BCP 2021

### ❖ Seismic Design Category (SDC)

- Contrary to BCP SP 2007, which used seismic zoning (from zone 1 to zone 4) to designate earthquake severity, the revised code now categorizes severity based on design category.
- The Seismic Design Category (SDC) is allocated to a structure depending on its occupancy category and the intensity of the design earthquake ground motion at the site.
- SDCs A, B, and C indicate lower hazard sites, whereas D, E, and F signify regions facing higher seismic risks.





# Introduction to BCP 2021

## □ Seismic Loading Criteria in BCP 2021

### ❖ Seismic Design Category (SDC)

- SDC is determined using Tables 1613.2.5(1) and 1613.2.5(2) of BCP-2021 (shown next) based on the design response spectral acceleration parameters ( $S_{DS}$  and  $S_s$ ) and the risk category of the structure.
- The risk category for buildings and other structures can be obtained Table 1604.5 of BCP-2021.



# Introduction to BCP 2021

## ❑ Seismic Loading Criteria in BCP 2021

### ❖ Seismic Design Category (SDC)

TABLE 1613.2.5(1)

SEISMIC DESIGN CATEGORY BASED ON SHORT-PERIOD (0.2 second) RESPONSE ACCELERATION

VALUE OF $S_{DS}$	RISK CATEGORY		
	I or II	III	IV
$S_{DS} < 0.167g$	A	A	A
$0.167g \leq S_{DS} < 0.33g$	B	B	C
$0.33g \leq S_{DS} < 0.50g$	C	C	D
$0.50g \leq S_{DS}$	D	D	D

TABLE 1613.2.5(2)

SEISMIC DESIGN CATEGORY BASED ON 1-SECOND PERIOD RESPONSE ACCELERATION

VALUE OF $S_{DI}$	RISK CATEGORY		
	I or II	III	IV
$S_{DI} < 0.067g$	A	A	A
$0.067g \leq S_{DI} < 0.133g$	B	B	C
$0.133g \leq S_{DI} < 0.20g$	C	C	D
$0.20g \leq S_{DI}$	D	D	D



# Introduction to BCP 2021

## ❑ Seismic Loading Criteria in BCP 2021

### ❖ Seismic Design Category (SDC)

- Seismic design categories of some districts of Pakistan for site class D and Seismic Risk Category III are provided below.

District	MCE RSA parameters		Site Coefficients		Adjusted MCE RSA parameters		DBE Parameters		SDC
	$S_s$	$S_1$	$F_a$	$F_v$	$S_{MS}$	$S_{M1}$	$S_{DS}$	$S_{D1}$	
Peshawar	0.84	0.29	1.164	2.02	0.98	0.59	0.65	0.39	D
Islamabad	1.3	0.38	1	1.92	1.30	0.73	0.87	0.49	D
Mansehra	1.17	0.36	1.032	1.94	1.21	0.70	0.80	0.47	D
Swat	1.06	0.40	1.076	1.9	1.14	0.76	0.76	0.51	D
Hangu	0.76	0.21	1.196	2.18	0.91	0.46	0.61	0.31	D
Mardan	0.76	0.32	1.196	1.98	0.91	0.63	0.61	0.42	D

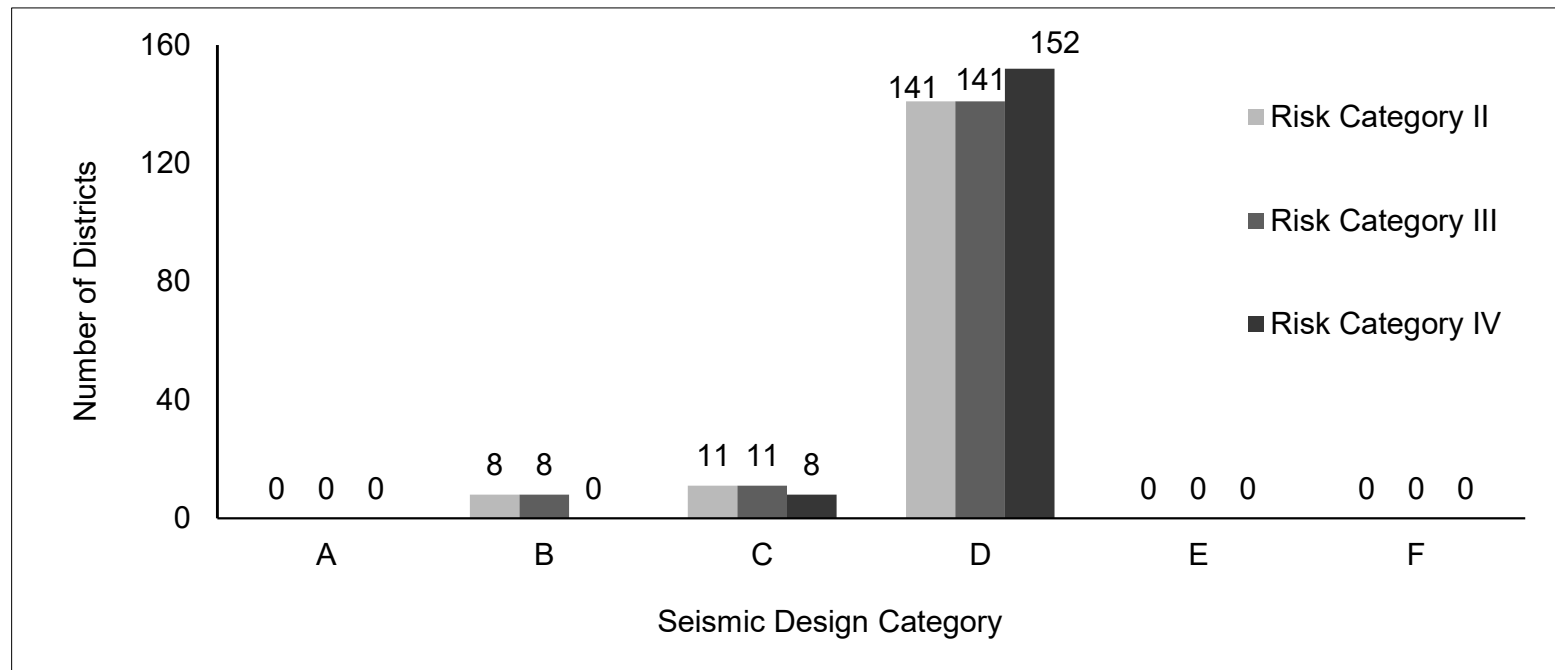


# Introduction to BCP 2021

## ❑ Seismic Loading Criteria in BCP 2021

### ❖ Seismic Design Category (SDC)

- The plot of districts in Pakistan versus seismic design categories (SDC) for different risk categories, considering site class as D, is shown in Figure below.



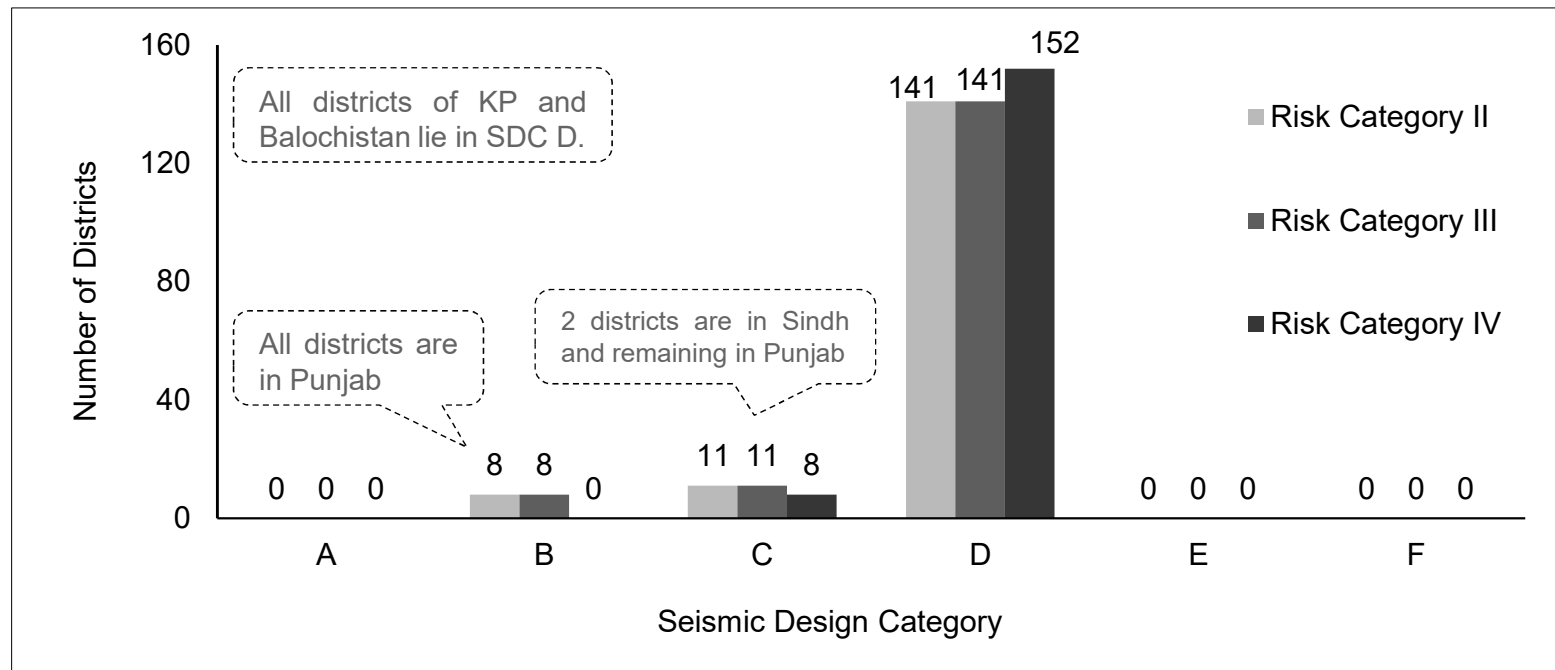


# Introduction to BCP 2021

## ❑ Seismic Loading Criteria in BCP 2021

### ❖ Seismic Design Category (SDC)

- The plot shows that out of 160 districts, no districts lie in SDC A, E and F. Only 8 districts fall in SDC B, and the majority are classified under SDC D.





# Introduction to BCP 2021

## ❑ Determination of Lateral Force

### ❖ Equivalent Lateral Force Procedure

- BCP 2021 refers to ASCE 7-16 Section 12.8 for determination of total design base shear force in a given direction.

$$V = C_s W$$

Where;

$$C_s = \text{seismic response coefficient} = \frac{S_{DS} I_e}{R}$$

- $S_{DS}$  = design response spectral acceleration parameter at short period
- $I_e$  = seismic importance factor, obtained from Table (1.5-2 of ASCE 7)
- $R$  = response modification factor, obtained from Table (12-2.1 of ASCE 7)

$W$  = effective seismic weight



# Introduction to BCP 2021

## □ Determination of Lateral Force

### ❖ Equivalent Lateral Force Procedure

#### ▪ Seismic Coefficient Limits

The value of  $C_s$  shall not exceed  $C_{s,max}$

$$C_{s,max} \begin{cases} \frac{S_{D1}I_e}{RT} & \text{for } T \leq T_L \\ \frac{S_{D1}I_eT_L}{RT^2} & \text{for } T > T_L \end{cases}$$

Where;

$S_{D1}$  = design response spectral acceleration parameter at long period

$T$  = structure's period (defined next)

$T_L$  = long-period transition period(s) determined from seismic maps (typically  $T_L = 8$  sec)

( $T_L$  marks the transition from the constant-velocity segment to the constant displacement segment of the design response spectrum)



# Introduction to BCP 2021

## □ Determination of Lateral Force

### ❖ Equivalent Lateral Force Procedure

#### ▪ Seismic Coefficient Limits

Similarly, the value of  $C_s$  shall not be less than  $C_{s,min}$

$$C_{s,min} = 0.044S_{DS}I_e \geq 0.01$$

In addition, for structures located where  $S_1$  is equal to or greater than  $0.6g$ ,  $C_s$  shall not be less than

$$C_{s,min} = \frac{0.5S_1I_e}{R}$$





# Introduction to BCP 2021

## □ Determination of Lateral Force

### ❖ Equivalent Lateral Force Procedure

#### ▪ Structure's Period

- The approximate fundamental period ( $T_a$ ), in seconds, shall be determined in accordance with ASCE 7, 12.8.2.1.

$$T_a = C_t(h_n)^x$$

Where;

- $h_n$  = structural height (from base level to top)
- $C_t = 0.028$  and  $x = 0.8$  (for steel moment-resisting frames)
- $C_t = 0.016$  and  $x = 0.9$  (for concrete moment-resisting frames)

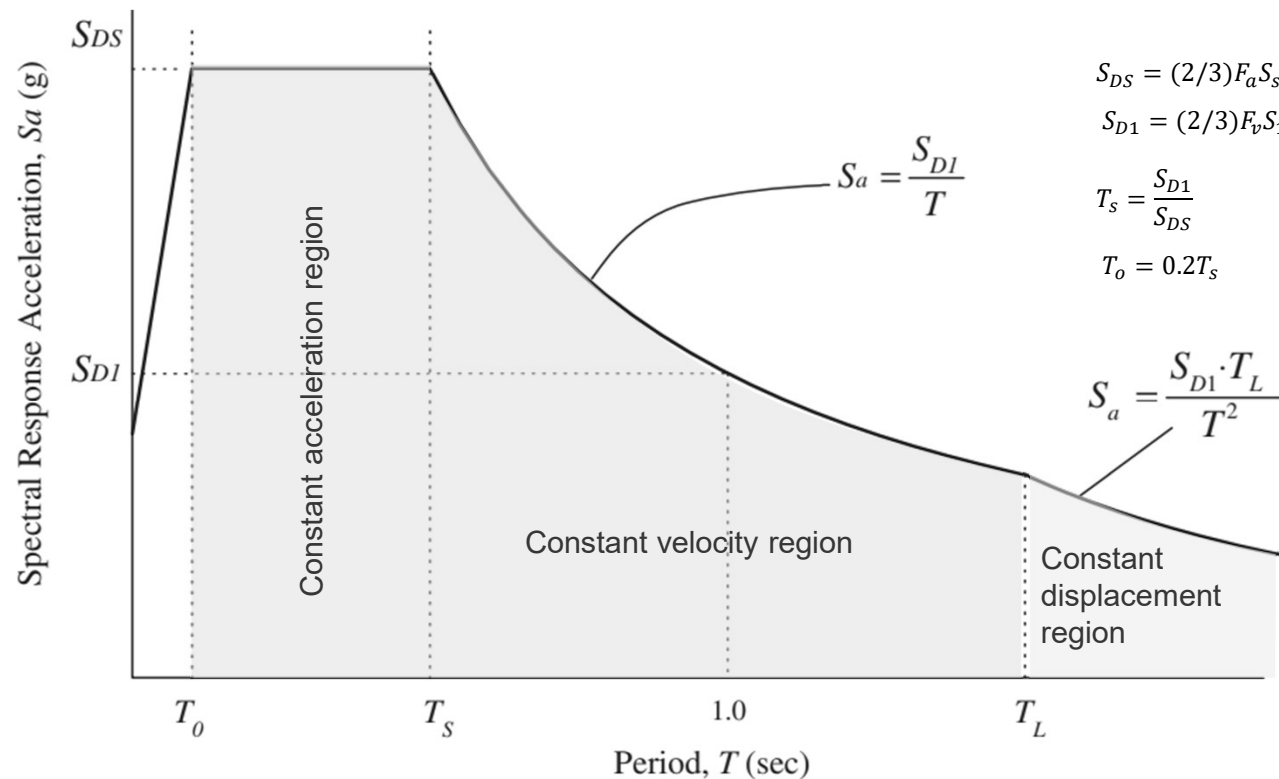


# Introduction to BCP 2021

## □ Determination of Lateral Force

### ❖ Dynamic Lateral Force Procedure

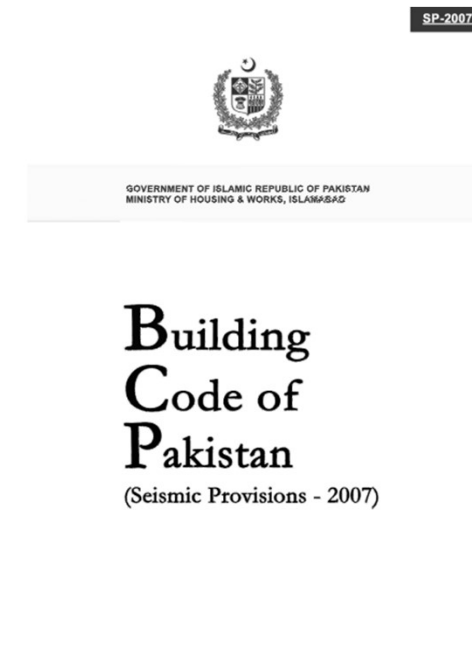
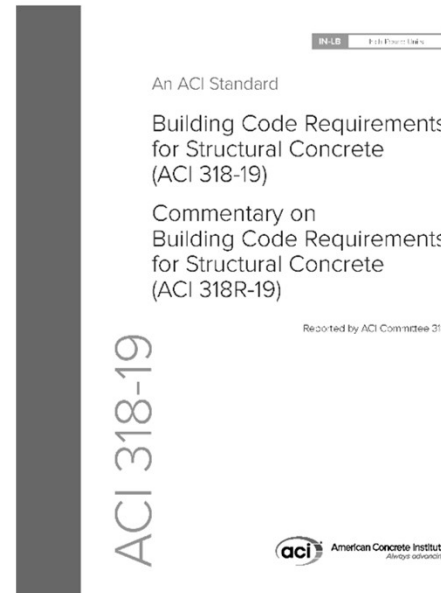
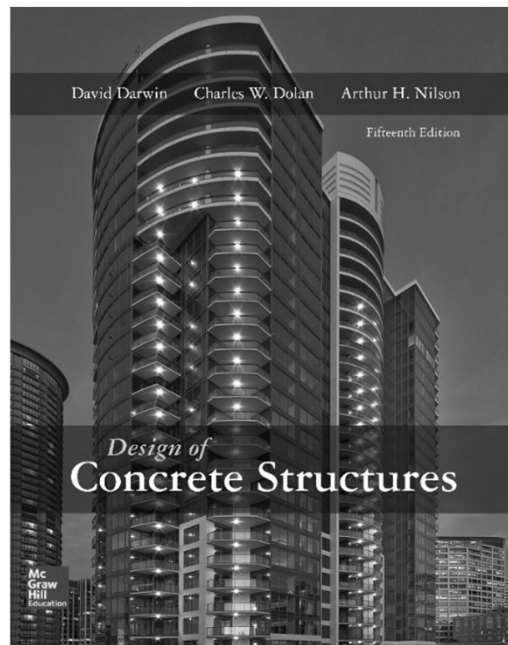
#### ▪ Response Spectrum Analysis





# References

- Design of Concrete Structures 14th / 15th edition by Nilson, Darwin and Dolan.
- Building Code Requirements for Structural Concrete (ACI 318-19)
- Building Code of Pakistan Seismic Provisions 2007 / UBC-97 Volume 2





# Appendix

**Table 2.2 — Seismic Zones of Tehsils of Pakistan**

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
<b>Punjab</b>					
<b>Attock</b>	2B	Shorkot	2A	<b>Multan City</b>	2A
Hassanabdal	2B	<b>Toba Tek Singh</b>	2A	Multan Saddar	2A
Fateh Jang	2B	Kamalia	2A	Shujabad	2A
Pindi Gheb	2B	Gojra	2A	Jalalpur Pirwala	2A
Jand	2B	<b>Gujranwala City</b>	2A	<b>Lodhran</b>	2A
<b>Rawalpindi</b>	2B	Wazirabad	2A	Kahrar Pacca	2A
Taxila	2B	Gujranwala Saddar	2A	Dunyapur	2A
Kahuta	2B	Nowshera Virkan	2A	<b>Khanewal</b>	2A
Murree	3	Kamoki	2A	Jehanian	2A
Kotli Sattian	3	<b>Hafizabad</b>	2A	Main Channu	2A
Gujar Khan	2B	Pindi Bhattian	2A	Kabirwala	2A
<b>Jhelum</b>	2B	<b>Gujrat</b>	2B	<b>Dera Ghazi Khan</b>	2A
Sohawa	2B	Kharian	2B	Taunsa	2B
Pind Dadan Khan	2B	Sarai Alamgir	2B	De-Ex.Area of D.G.Khan	2B
Dina	2B	<b>Mandi Bahauddin</b>	2B	<b>Rajanpur</b>	2A
<b>Chakwal</b>	2B	Malikwal	2B	Rojhan	2A
Talagang	2B	Phalia	2A	Jampur	2A
Choa Saidan Shah	2B	<b>Sialkot</b>	2B	De-Ex.Area of Rajanpur	2B



# Appendix

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
<b>Sargodha</b>	2A	Daska	2B	<b>Leiah</b>	2A
Sillanwali	2A	Pasrur	2B	Chaubara	2A
Bhalwal	2A	<b>Narowal</b>	2B	Karor Lal Esan	2A
Shahpur	2B	Shakargarh	2B	<b>Muzaffargarh</b>	2A
Sahiwal	2A	<b>Lahore</b> City	2A	Alipur	2A
Kot Momin	2A	Lahore Cantt	2A	Jatoi	2A
<b>Bhakkar</b>	2A	<b>Kasur</b>	2A	Kot Addu	2A
Kalur Kot	2B	Chunian	2A	<b>Bahawalpur</b>	2A
Mankera	2A	Pattoki	2A	Hasilpur	2A
Darya Khan	2A	<b>Okara</b>	2A	Yazman	2A
<b>Khushab</b>	2B	Depalpur	2A	Ahmadpur East	2A
Nurpur	2A	Renala Khurd	2A	Khairpur Tamewali	2A
<b>Mianwali</b>	2B	<b>Sheikhupura</b>	2A	<b>Bahawalnagar</b>	2A
Isa Khel	2B	Nankana Sahib	2A	Minchinabad	2A
Piplan	2B	Ferozwala	2A	Fort Abbas	1
<b>Faisalabad</b> City	2A	Safdarabad	2A	Haroonabad	2A
Faisalabad Saddar	2A	<b>Vehari</b>	2A	Chishtian	2A
Chak Jhumra	2A	Burewala	2A	<b>Rahim Yar Khan</b>	2A
Sammundri	2A	Mailsi	2A	Khanpur	2A
Jaranwala	2A	<b>Sahiwal</b>	2A	Liaquatpur	2A
Tandlianwala	2A	Chichawatni	2A	Sadiqabad	2A
<b>Jhang</b>	2A	<b>Pakpattan</b>	2A		
Chiniot	2A	Arifwala	2A		



# Appendix

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
<b>Balochistan</b>					
<b>Quetta</b>	3	<b>Dera Bugti</b>	3	Aranji (S/T)	2B
Panjpai (S/T)	3	Sangsillah (S/T)	3	<b>Awaran</b>	2B
<b>Pishin</b>	4	Sui	3	Mshki (S/T)	3
Hurramzai (S/T)	4	Loti	3	Jhal Jao	3
Barshore (S/T)	3	Phelawagh	3	<b>Kharan</b>	3
Karezat (S/T)	4	Malam (S/T)	3	Besima (S/T)	2B
Bostan (S/T)	4	Baiker (S/T)	3	Nag (S/T)	2B
<b>Killa Abdullah</b>	3	Pir Koh (S/T)	3	Wasuk (S/T)	2B
Gulistan (S/T)	3	<b>Jaffarabad/Jhat Pat</b>	2B	Mashkhel (S/T)	2A
Chaman	3	Panhwar (S/T)	2B	<b>Bela</b>	2B
Dobandi (S/T)	3	Usta Mohammad	2B	Uthal	2B
<b>Chagai (S/T)</b>	2A	Gandaka (S/T)	2B	Lakhra	2B
Dalbandin	2A	<b>Nasirabad/Chattar</b>	3	Liari (S/T)	2B
Nushki	4	Tamboo	3	Hub	2B
Nokundi S/T	2A	D.M.Jamali	2B	Gadani (S/T)	2B
Taftan	2A	<b>Bolan/Dhadar</b>	3	Sonmiani/Winder	2B
<b>Loralai/Bori</b>	3	Bhag	3	Dureji	2B
Mekhtar (S/T)	3	Balanari (S/T)	3	Kanraj	2B
Duki	3	Sani (S/T)	3	<b>Kech</b>	2B
<b>Barkhan</b>	3	Khattan (S/T)	3	Buleda (S/T)	2B
<b>Musakhel</b>	3	Mach	3	Zamuran (S/T)	2B



# Appendix

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
Kingri (S/T)	3	<b>Kachhi</b> /Gandawa	2B	Hoshab (S/T)	2B
<b>Killa Saifullah</b>	3	Mirpur (S/T)	2B	Balnigor (S/T)	2B
Muslim Bagh	4	Jhal Magsi	2B	Dasht (S/T)	3
Loiband (S/T)	3	<b>Kalat</b>	3	Tump	2B
Baddini (S/T)	3	Mangochar (S/T)	3	Mand (S/T)	2B
<b>Zhob</b>	3	Johan (S/T)	3	<b>Gwadar</b>	3
Sambaza (S/T)	3	Surab	2B	Jiwani	2B
Sherani (S/T)	3	Gazg (S/T)	3	Suntsar (S/T)	2B
Qamar Din Karez	2B	<b>Mastung</b>	3	Pasni	3
Ashwat (S/T)	2B	Kirdgap (S/T)	3	Ormara	3
<b>Sibi</b>	3	Dasht	3	<b>Panjgur</b>	2B
Kutmandai (S/T)	3	Khad Koocha (S/T)	3	Parome (S/T)	2B
Sangan (S/T)	3	<b>Khuzdar</b>	2B	Gichk (S/T)	2B
Lehri	3	Zehri	2B	Gowargo	2A
<b>Ziarat</b>	4	Moola (S/T)	2B		
Harnai	3	Karakh (S/T)	2B		
Sinjawi (S/T)	4	Nal (S/T)	3		
<b>Kohllu</b>	3	Wadh (S/T)	2B		
Kahan	3	Ornach (S/T)	3		
Mawand	3	Saroon (S/T)	2B		



# Appendix

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
<b>NWFP</b>					
<b>Chitral</b>	4	<b>Swabi</b>	2B	<b>Kurram</b>	
Drosh	3	Lahore	2B	Lower <b>Kurram</b>	2B
Lutkoh	3	<b>Charsadda</b>	2B	Upper Kurram	2B
Mastuj	3	Tangi	3	Kurram F.R.	2B
Turkoh	3	<b>Peshawar</b>	2B	<b>Orakzai</b>	
Mulkoh	3	<b>Nowshera</b>	2B	Central <b>Orakzai</b>	2B
<b>Dir</b>	3	<b>Kohat</b>	2B	Lower Orkzai	2B
Barawal	3	Lachi	2B	Upper Orkzai	2B
Kohistan	3	<b>Hangu</b>	2B	Ismailzai	2B
Wari	3	<b>Karak</b>	2B	<b>South Waziristan</b>	
Khall	3	Banda Daud Shah	2B	Ladha	2B
Temergara	3	Takht-E-Nasrati	2B	Makin (Charlai)	2B
Balambat	3	<b>Bannu</b>	2B	Sararogha	2B
Lalqila	3	<b>Lakki Marwat</b>	2B	Sarwekai	2B
Adenzai	3	<b>Dera Ismail Khan</b>	2A	Tiarza	2B
Munda	3	Daraban	3	Wana	2B
Samarbagh (Barwa)	3	Paharpur	2B	Toi Khullah	2B
<b>Swat</b>		Kulachi	2B	Birmal	2B
Matta	3	<b>Tank</b>	2B	<b>North Waziristan</b>	
<b>Shangla/Alpuri</b>	3	<b>Bajaur</b>		Datta Khel	2B





# Appendix

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
Besham	3	Barang	3	Dossali	2B
Chakesar	3	Charmang	3	Garyum	2B
Martung	3	Khar Bajaur	3	Ghulam Khan	2B
Puran	2B	Mamund	3	Mir Ali	2B
<b>Buner/Daggar</b>	2B	Salarzai	3	Miran Shah	2B
<b>Malakand/Swat</b>					
Ranizai	3	Utmankhel (Qzafi)	3	Razmak	2B
Sam Ranizai	2B	Nawagai	3	Spinwam	2B
<b>Dassu</b>	3	<b>Mohmand</b>		Shewa	2B
Pattan	3	Halimzai	3		
Palas	3	Pindiali	3		
<b>Mansehra</b>	3	Safi	3		
Balakot	4	Upper Mohmand	3		
Oghi	2B	Utman Khel(Ambar)	3		
T.A.Adj.Mansehra Distt	3	Yake Ghund	3		
<b>Batagram</b>	3	Pringhar	3		
Allai	3	<b>Khyber</b>			
<b>Abbottabad</b>	3	Bara	2B		
<b>Haripur</b>	2B	Jamurd	2B		
Ghazi	2B	Landi Kotal	3		
<b>Mardan</b>	2B	Mula Ghor	3		
Takht Bhai	2B				



# Appendix

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
<b>Sindh</b>					
<b>Jacobabad</b>	2A	Khairpur Nathan Shah	2B	<b>Tharparkar/Chachro</b>	2A
Garhi Khairo	2A	Sehwan	2A	Nagar Parkar	2B
Thul	2A	Mehar	2A	Diplo	3
Kandhkot	2A	Johi	2B	Mithi	2B
Kashmor	2A	Kotri	2A	<b>Karachi East</b>	2B
<b>Shikarpur</b>	2A	Thano Bula Khan	2A	<b>Karachi West</b>	2B
Khanpur	2A	<b>Hyderabad City</b>	2A	<b>Karachi South</b>	2B
Garhi Yasin	2A	Matiori	2A	<b>Karachi Central</b>	2B
Lakhi	2A	Tando Allahyar	2A	<b>Malir</b>	2B
<b>Larkana</b>	2A	Hala	2A		
Miro Khan	2A	Latifabad	2A	<b>FEDERAL AREA</b>	
Rato Dero	2A	Hyderabad	2A	Islamabad	2B
Shahdadkot	2B	Qasimabad	2A		
Dokri	2A	Tando Mohd Khan	2A	<b>AJK</b>	
Kambar	2B	<b>Badin</b>	2B	Bagh	4
Warah	2A	Golarchi	2A	Bhimbar	2B
<b>Sukkur</b>	2A	Matli	2A	Hajira	4
Rohri	2A	Tando Bagho	2B	Kotli	3
Pano Aqil	2A	Talhar		Muzaffarabad	4
Salehpat	2A	<b>Thatta</b>	2A	New Mirpur	2B



# Appendix

Tehsil	Seismic Zone	Tehsil	Seismic Zone	Tehsil	Seismic Zone
<b>Ghotki</b>	2A	Mirpur Sakro	2A	Palandri	3
Khangarh	2A	Keti Bunder	2A	Rawalakot	3
Mirpur Mathelo	2A	Ghorabari	2A		
Ubauro	2A	Sujawal	2A	<b>NORTHERN AREA</b>	
Daharki	2A	Mirpur Bathoro	2A	Chilas	3
<b>Khairpur</b>	2A	Jati	2A	Dasu	3
Kingri	2A	Shah Bunder	2A	Gakuch	3
Sobhoderro	2A	Kharo Chan	2A	Gilgit	3
Gambat	2A	<b>Sanghar</b>	2A	Ishkuman	2B
Kot Diji	2A	Sinjhoro	2A	Skardu	3
Mirwah	2A	Khipro	2A	Yasin	3
Faiz Ganj	2A	Shahdadpur	2A		
Nara	2A	Jam Nawaz Ali	2A		
<b>Naushahro Feroze</b>	2A	Tando Adam	2A		
Kandioro	2A	<b>Mirpur Khas</b>	2A		
Bhiria	2A	Digri	2A		
Moro	2A	Kot Ghulam Moh	2A		
<b>Nawab Shah</b>	2A	<b>Umerkot</b>	2A		
Skrand	2A	Samaro	2A		
Daulatpur	2A	Kunri	2A		
<b>Dadu</b>	2A	Pithoro	2A		



# Appendix

RISK CATEGORY	NATURE OF OCCUPANCY
I	Buildings and other structures that represent a low hazard to human life in the event of failure, including but not limited to: <ul style="list-style-type: none"> <li>Agricultural facilities, Certain temporary facilities and Minor storage facilities</li> </ul>
II	Buildings and other structures except those listed in Risk Categories I, III and IV
III	Buildings and other structures that represent a substantial hazard to human life in the event of failure, including but not limited to: <ul style="list-style-type: none"> <li>Buildings and other structures whose primary occupancy is public assembly with an occupant load greater than 300.</li> <li>Buildings and other structures containing one or more public assembly spaces, each having an occupant load greater than 300 and a cumulative occupant load of the public assembly spaces of greater than 2,500.</li> <li>Buildings and other structures containing Group E or Group I-4 occupancies or combination thereof, with an occupant load greater than 250.</li> <li>Buildings and other structures containing educational occupancies for students above the 12th grade with an occupant load greater than 500.</li> <li>Group I-2, Condition 1 occupancies with 50 or more care recipients.</li> <li>Group I-2, Condition 2 occupancies not having emergency surgery or emergency treatment facilities.</li> <li>Group I-3 occupancies.</li> <li>Any other occupancy with an occupant load greater than 5,000.</li> <li>Power-generating stations, water treatment facilities for potable water, wastewater treatment facilities and other public utility facilities not included in Risk Category IV.</li> <li>Buildings and other structures not included in Risk Category IV containing quantities of toxic or explosive materials that are sufficient to pose a threat to the public if released.</li> </ul>
IV	Buildings and other structures designated as essential facilities, including but not limited to: <ul style="list-style-type: none"> <li>Group I-2, Condition 2 occupancies having emergency surgery or emergency treatment facilities.</li> <li>Ambulatory care facilities having emergency surgery or emergency treatment facilities.</li> <li>Fire, rescue, ambulance and police stations and emergency vehicle garages</li> <li>Designated earthquake, hurricane or other emergency shelters.</li> <li>Designated emergency preparedness, communications and operations centers and other facilities required for emergency response.</li> <li>Power-generating stations and other public utility facilities required as emergency backup facilities for Risk Category IV structures.</li> <li>Buildings and other structures containing quantities of highly toxic materials that are sufficient to pose a threat to the public if released.</li> <li>Aviation control towers, air traffic control centers and emergency aircraft hangars.</li> <li>Buildings and other structures having critical national defense functions.</li> <li>Water storage facilities and pump structures required to maintain water pressure for fire suppression.</li> </ul>

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